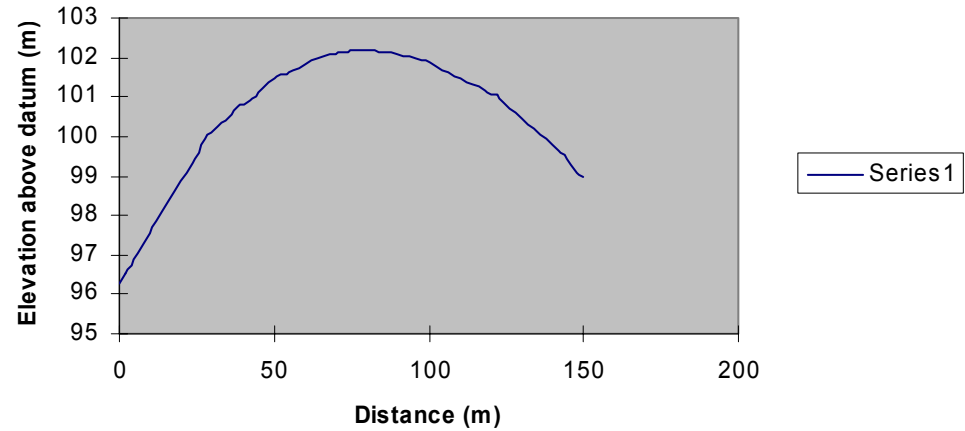
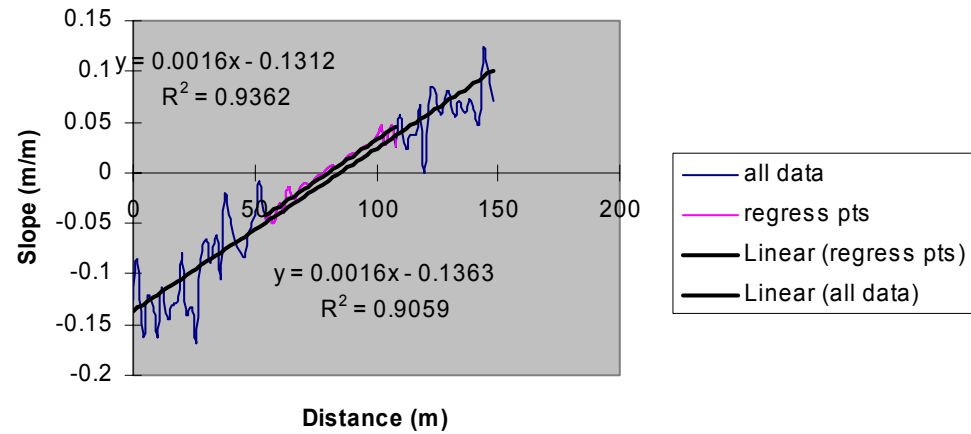


### 990k Undisturbed Pavement Ridge

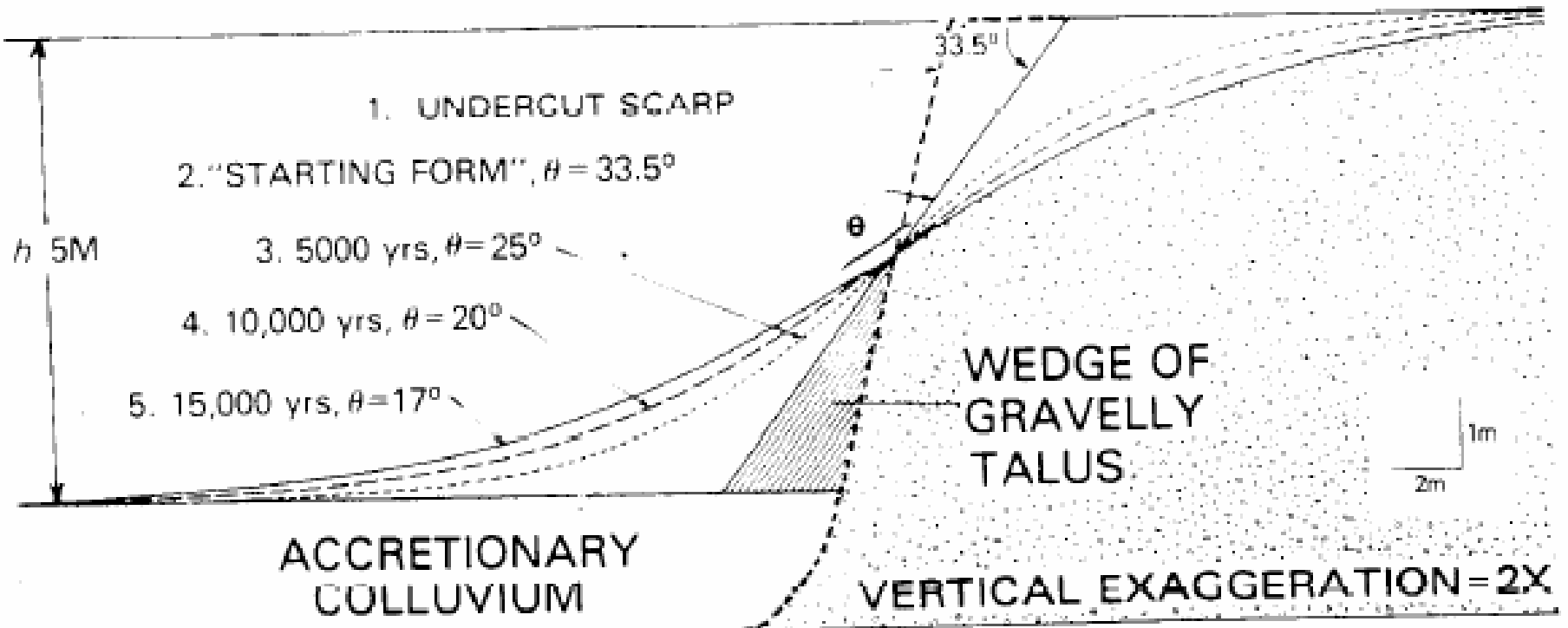


### 990k Undisturbed Pavement Ridge

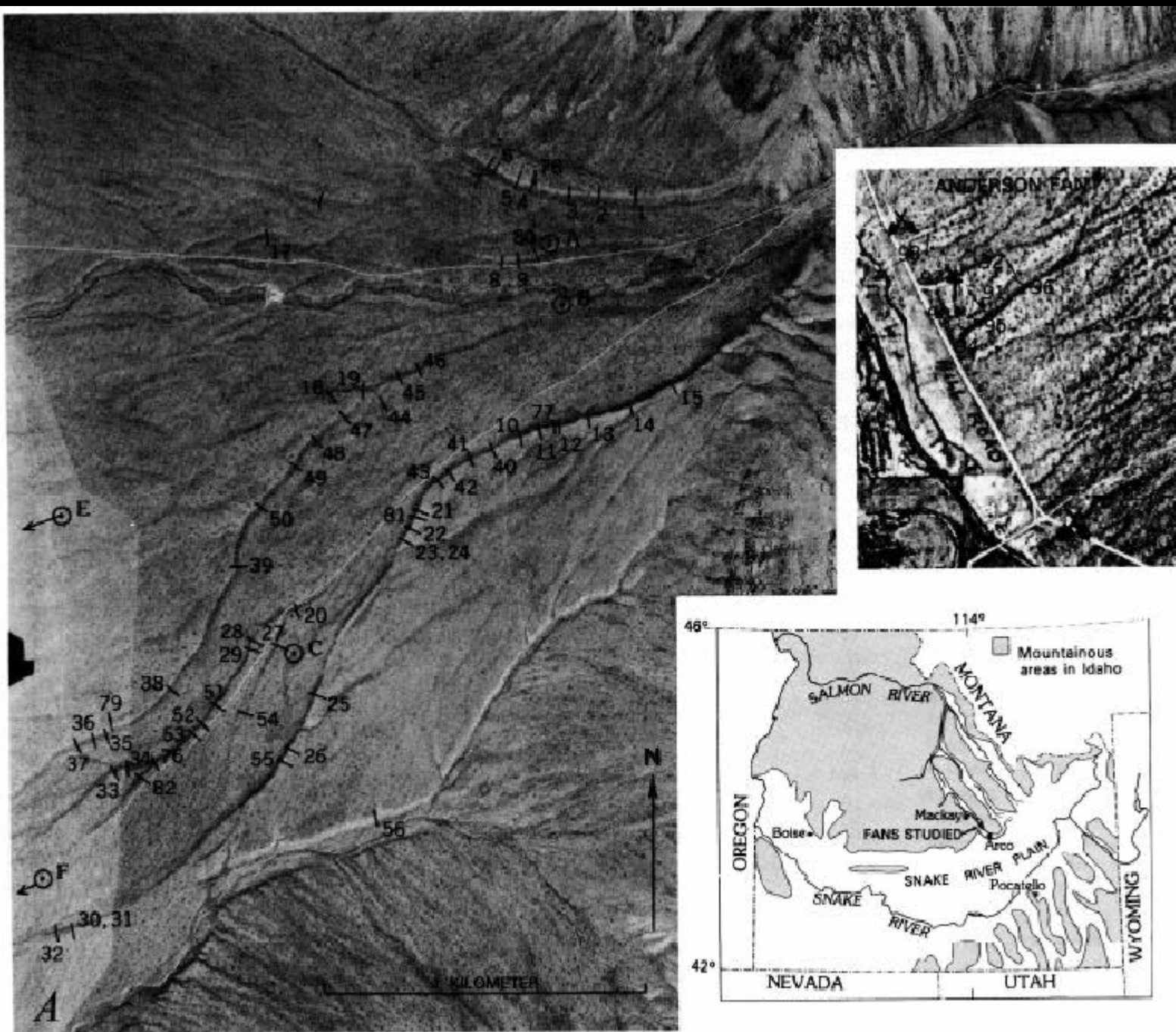


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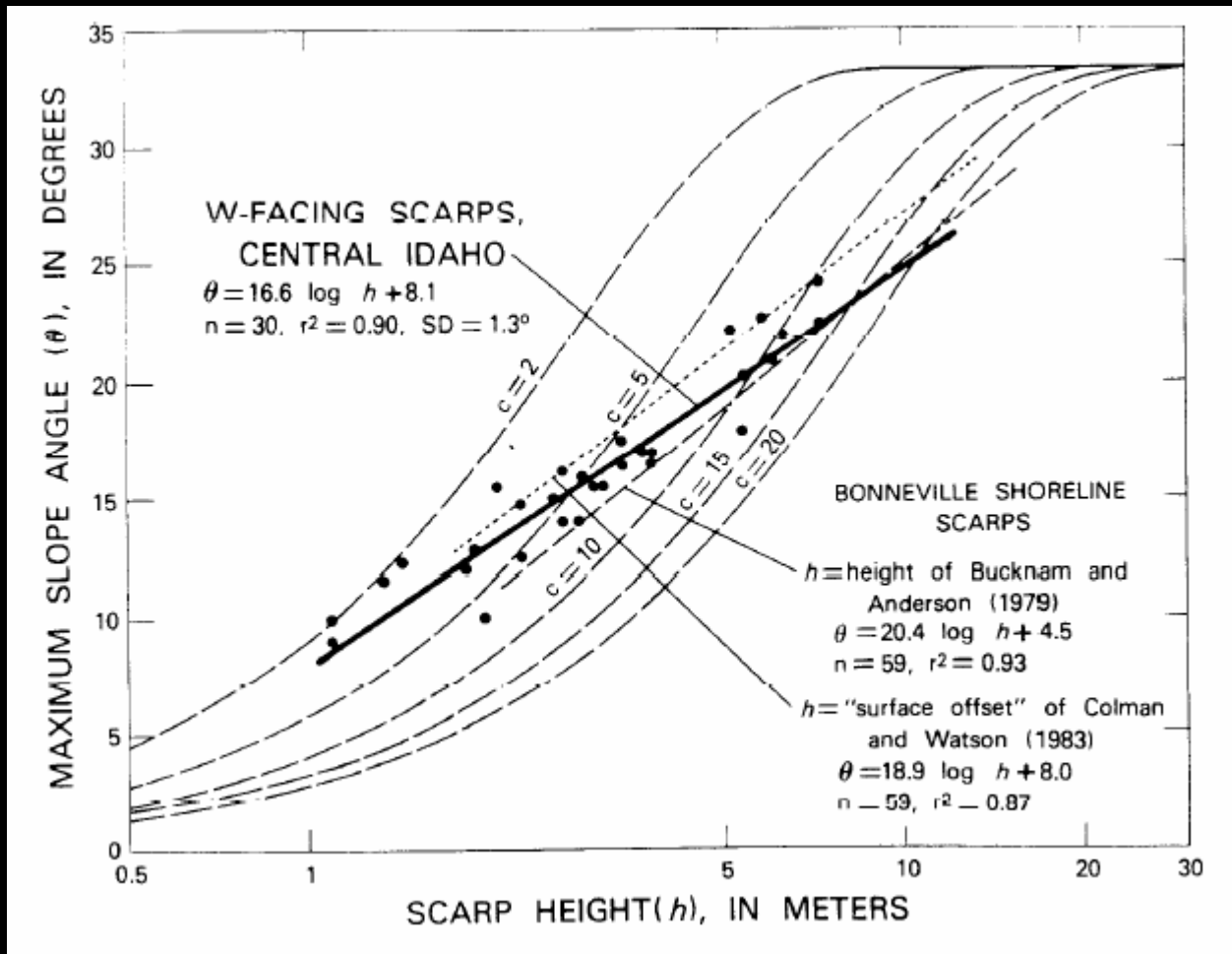
Geological Society of America Bulletin, v. 97, p. 869-885, 15 figs., 5 tables, July 1986.



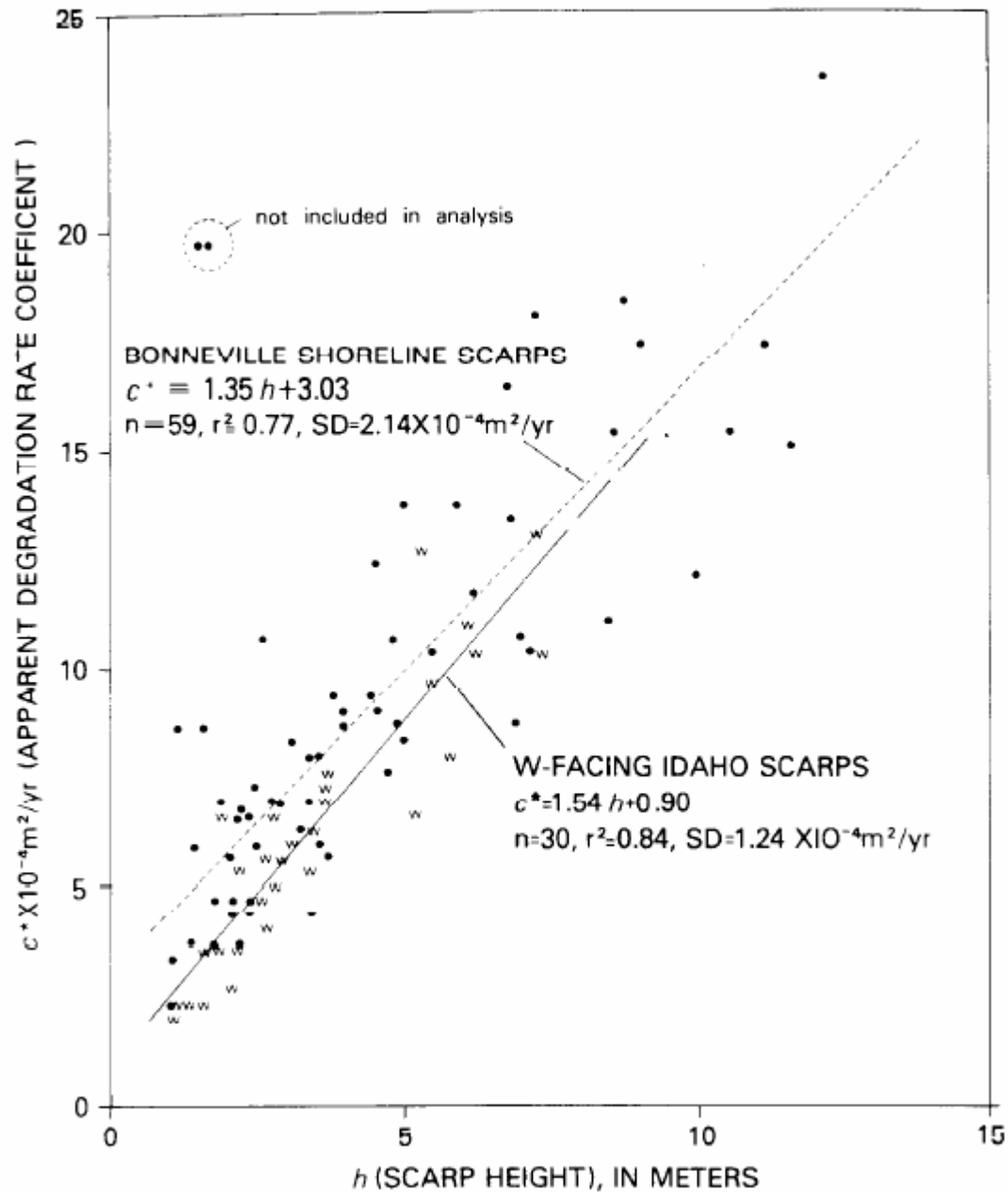
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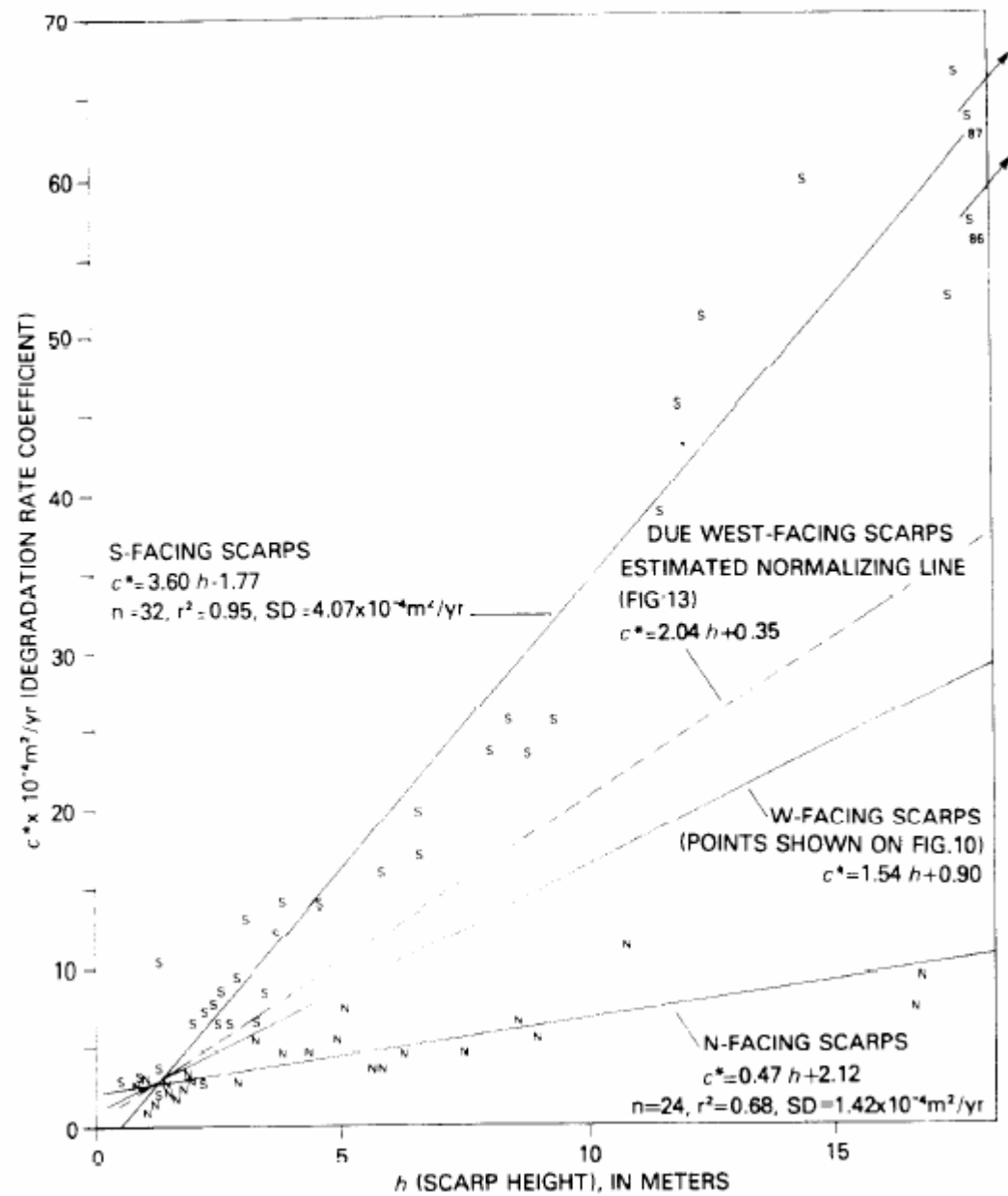
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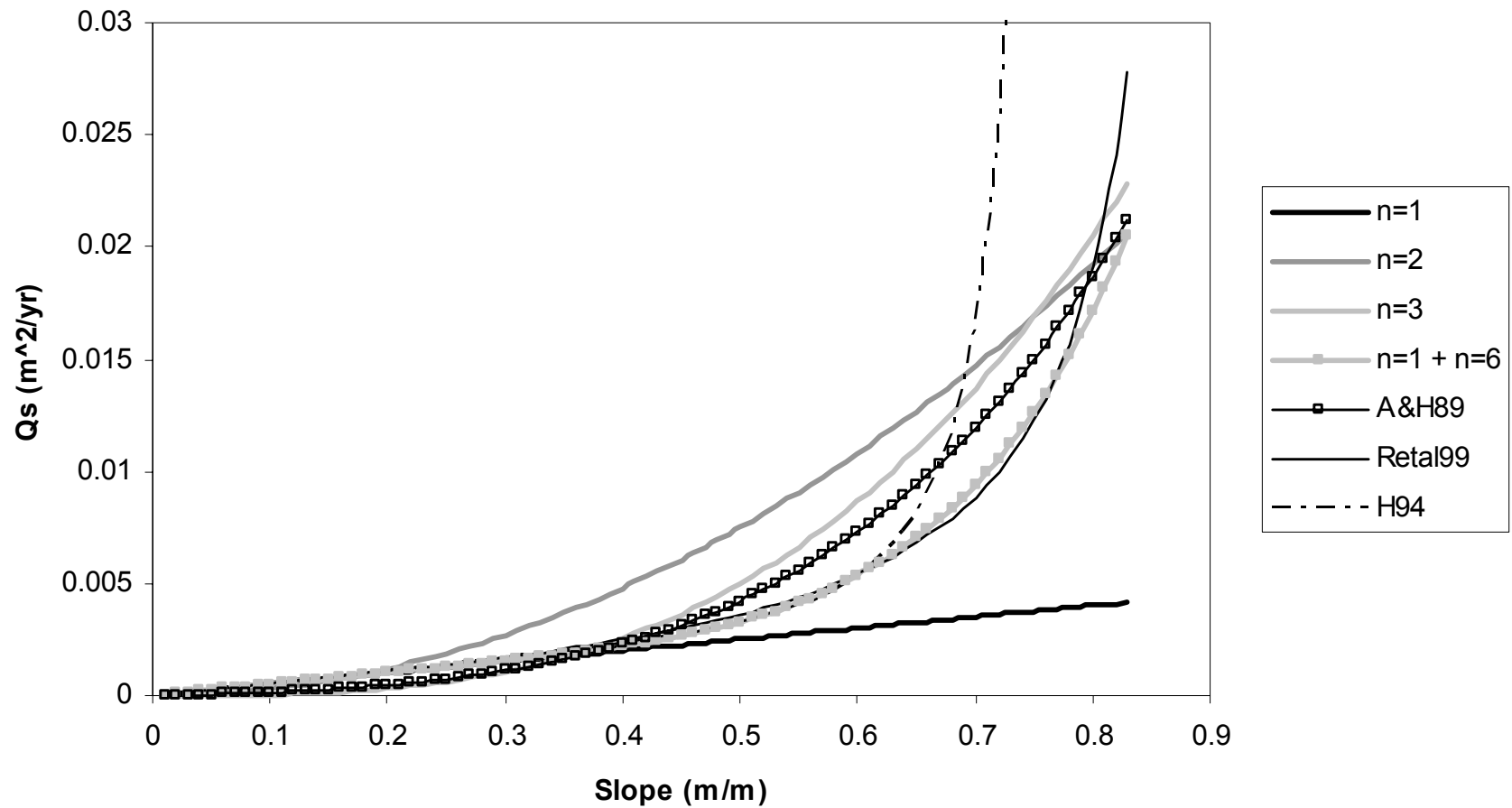
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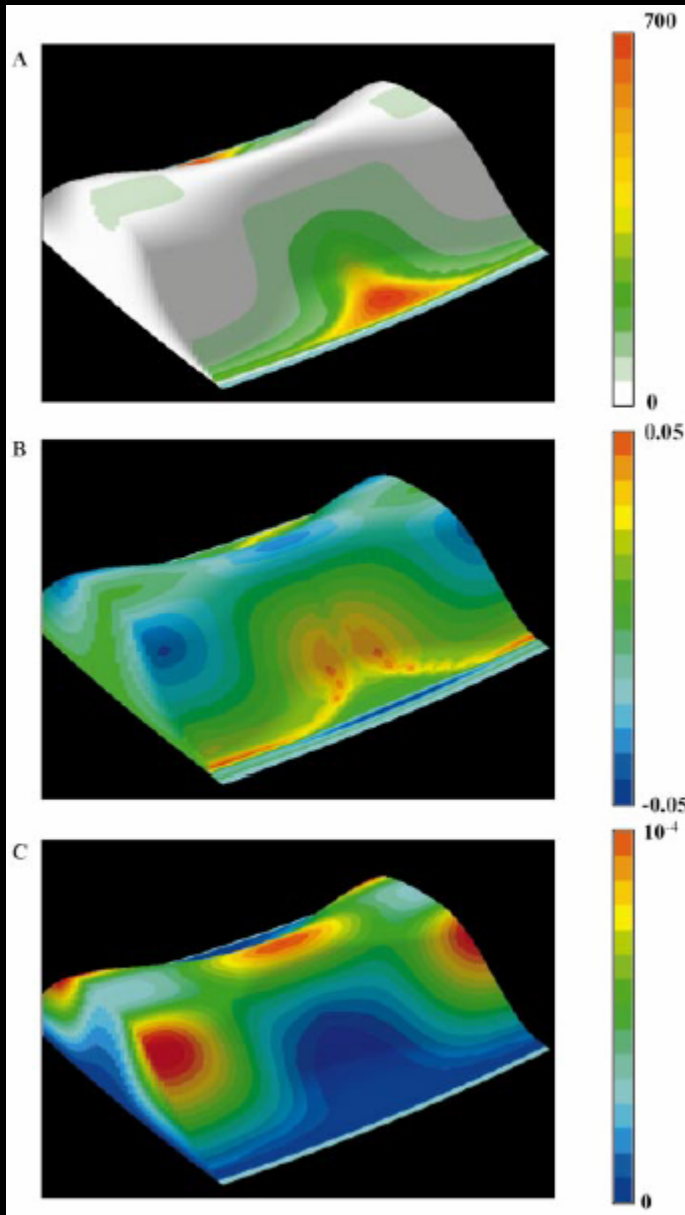


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soil depth

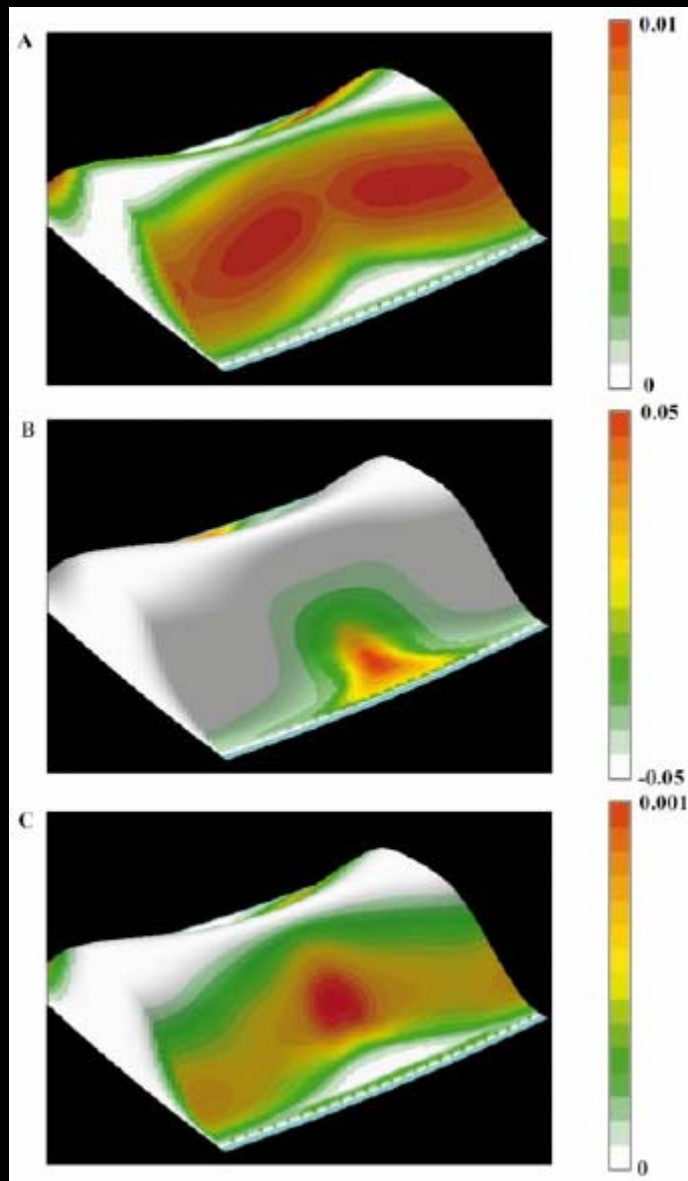
Steady State Condition

curvature

soil production rate

Figure 1. Results of model after 100 k.y., in form of (A) soil thickness (in centimeters), (B) surface curvature (per meter) calculated at resolution of numerical grid (10 m), and (C) soil production rate (in meters per year). Transport parameter values (see text) are:  $K_D = 3 \times 10^{-3} \text{ m}^2 \cdot \text{yr}^{-1}$ ,  $K_v = 3 \times 10^{-3} \text{ m}^2 \cdot \text{m}^{-1} \cdot \text{yr}^{-1}$ ,  $K_w = 3 \times 10^{-9} \text{ m}^2 \cdot \text{yr}^{-1} \cdot \text{m}^{-3k}$ ,  $m = 1.67$ ,  $n = 0.5$ ,  $k = 1.67$ ,  $p = 1.3$ ,  $P_0 = 53 \times 10^{-6} \text{ m} \cdot \text{yr}^{-1}$ ,  $h_0 = 0.5 \text{ m}$ . The precipitation rate is assumed to be uniform at  $0.5 \text{ m} \cdot \text{yr}^{-1} \cdot \kappa$ ; the ratio of soil to bedrock density is 0.5. The time step used in the explicit time integration is 10 yr.





qs – linear creep

qs – rheologic creep

qs -- sheetwash

Figure 2. Contributions to total local soil flux (in  $\text{m}^2\cdot\text{yr}^{-1}$ ) from (A) linear creep, (B) depth-dependent creep, and (C) overland flow as computed by model after 100 k.y. Parameters are same as for Figure 1.

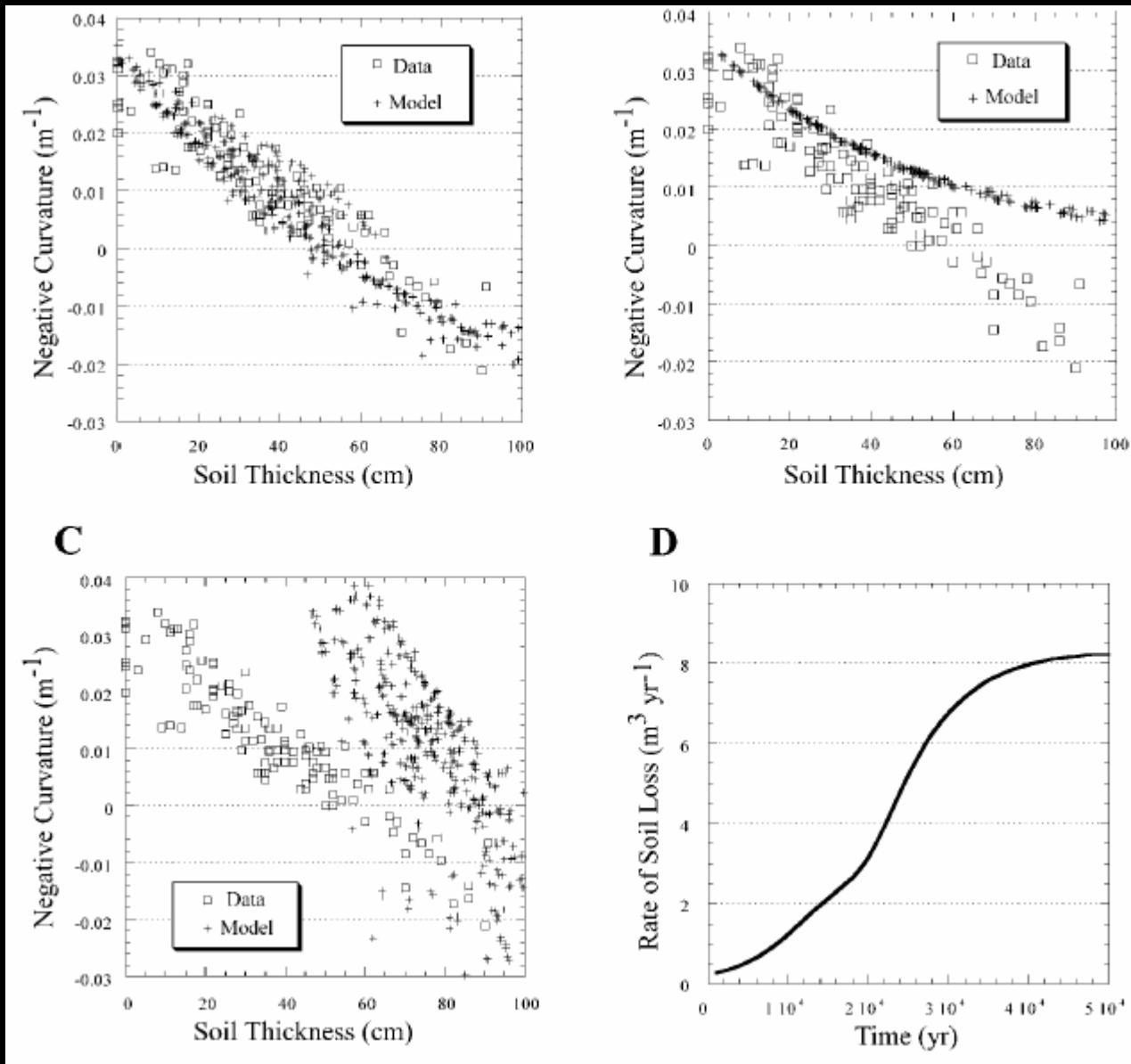


Figure 3. Computed curvature-thickness curves compared to observations at Bega Valley site (Heimsath et al., 2000) by using (A) model parameters given in Figure 1 caption (see text), (B) with  $K_V$  and  $K_W$  both set to zero, and (C)  $K_D$  and  $K_W$  set to zero. Rate of soil loss through upper and lower open boundaries is given in D for set of parameters given in Figure 1 caption.

# Quantification of soil production and downslope creep rates from cosmogenic $^{10}\text{Be}$ accumulations on a hillslope profile

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$$\int_0^x P_{\text{Be}}(x) dx = \int_0^h \rho_s(x, z) V_s(x, z) \epsilon_{\text{Be}}(x, z) dz.$$

$$Q_s(x) = \frac{P_{\text{Be}} x}{\rho_s C_{\text{Be}}(x)}.$$

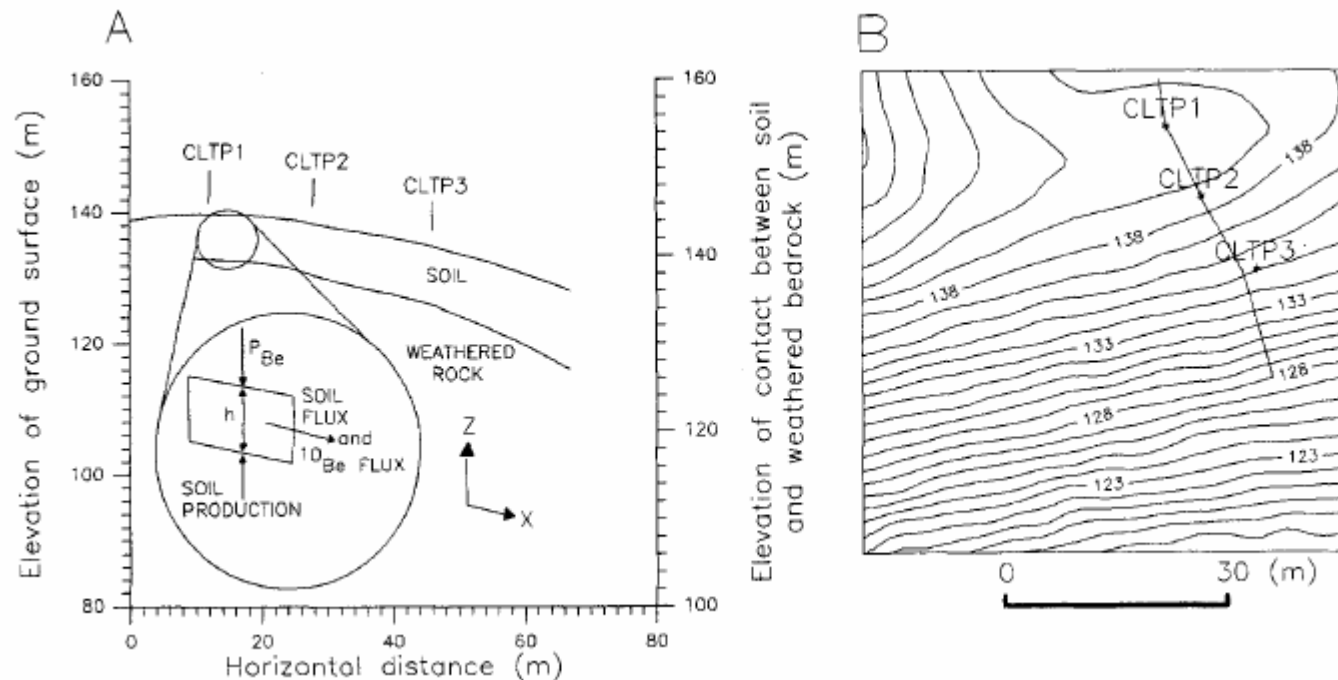


Figure 1. A: Hillslope profile (see B for location) with soil horizon defined on vertically exaggerated right-hand depth scale. Inset figure shows  $^{10}\text{Be}$  and soil mass balance over some slope profile length interval  $x$ .  $P_{\text{Be}}$  = hillslope  $^{10}\text{Be}$  deposition rate; soil creep mass flux =  $\rho_s V_s h$ ;  $^{10}\text{Be}$  mass flux =  $C_{\text{Be}} \rho_s V_s h$ ; and soil production rate =  $\rho_s C_o$ . B: Black Diamond site topography. CLTP1, CLTP2, and CLTP3 are  $^{10}\text{Be}$  sample locations on slope profile. Contour interval = 1 m. Scale bar = 30 m.

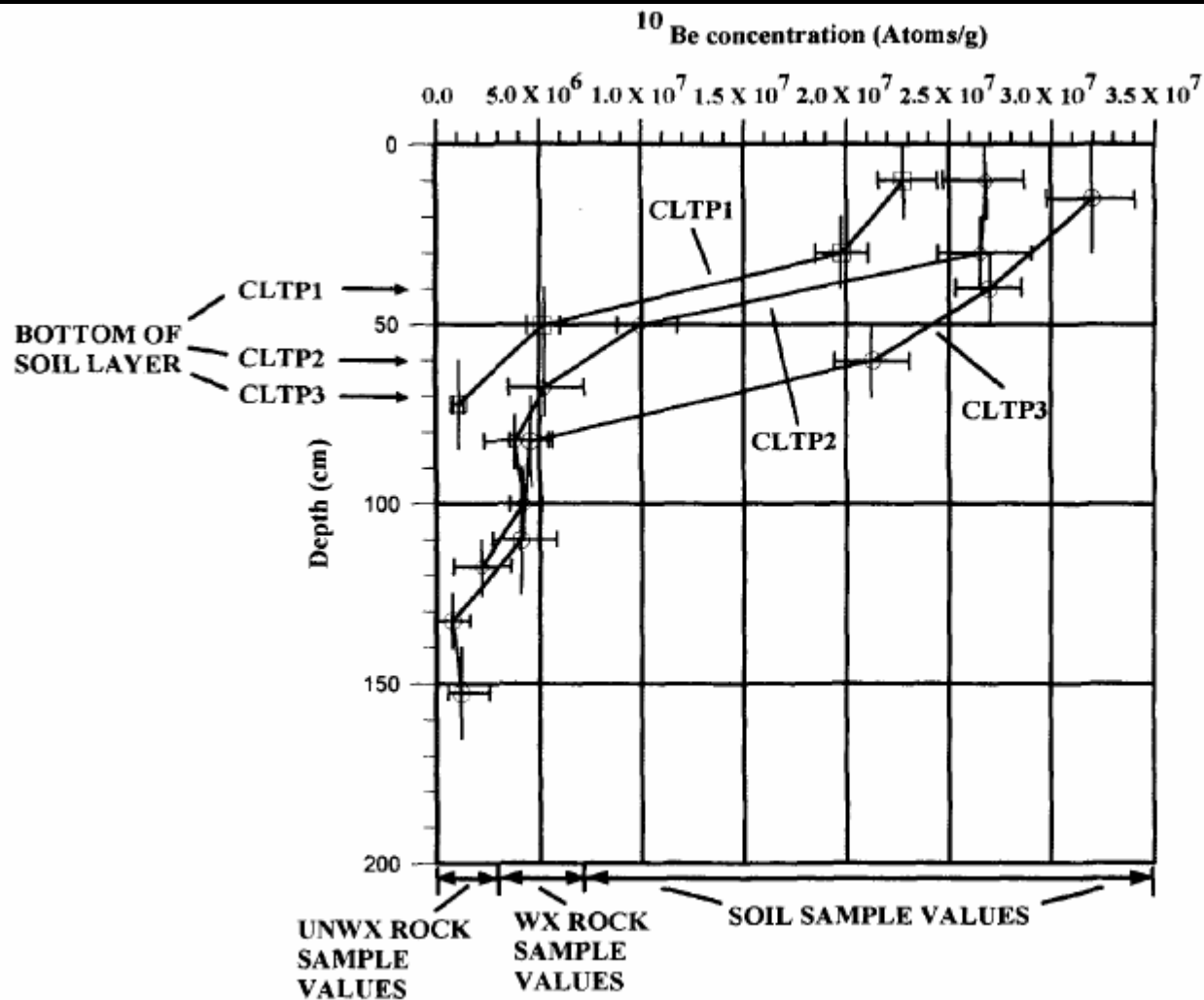
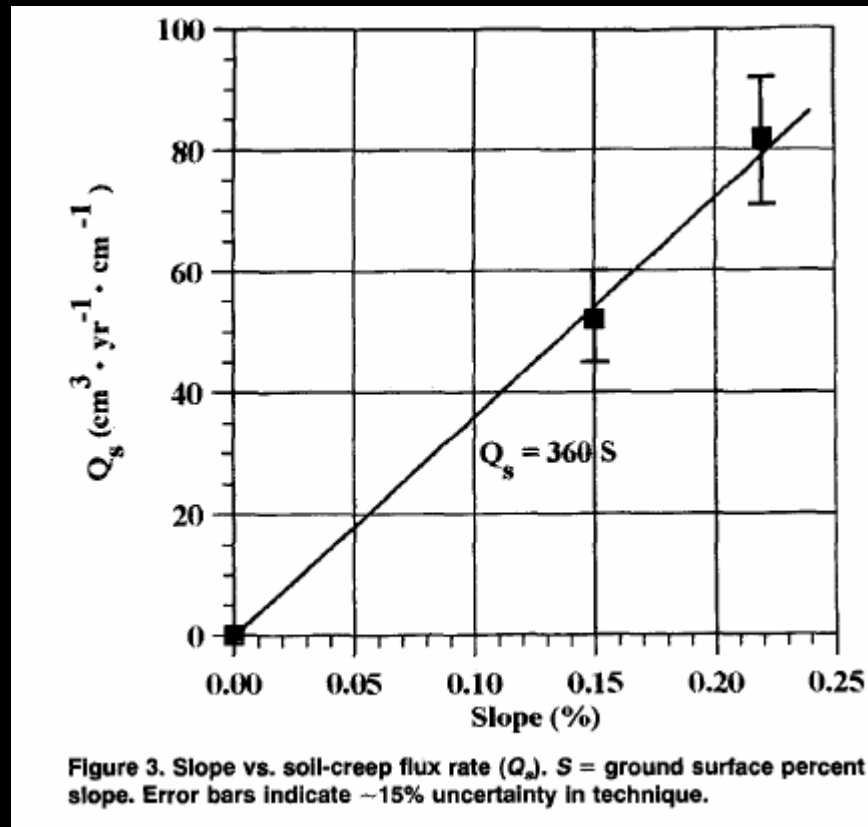


Figure 2. Profiles of  $^{10}\text{Be}$  concentration in test pits, Black Diamond Mines Regional Preserve. Vertical lines through data points represent sample depth intervals. Each data point is average of two duplicate measurements. Error bars represent maximum and minimum AMS  $1\sigma$  uncertainty values for two duplicate samples. WX = weathered bedrock and UNWX = unweathered bedrock. Bottom of soil horizon in each test pit noted on vertical axis.

$$Q_s(x) = \frac{P_{Be}x}{\rho_s C_{Be}(x)}$$



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Confirmation  $Q_s$  linear with Slope (over this range of shallow slopes); but  
 No direct test of (a) non-linearity at high slopes, (b) dependence on soil depth