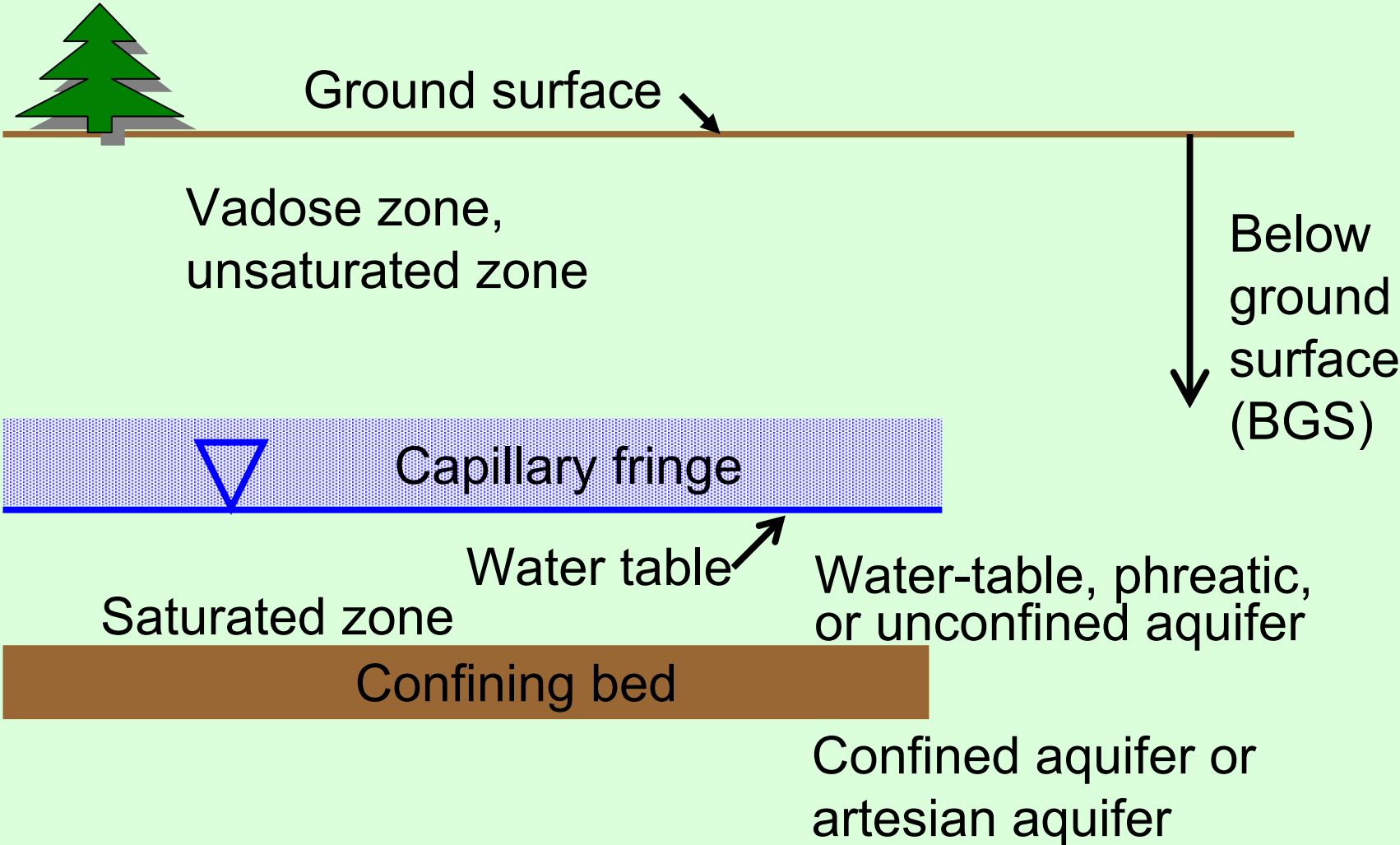


# **Lecture 3**

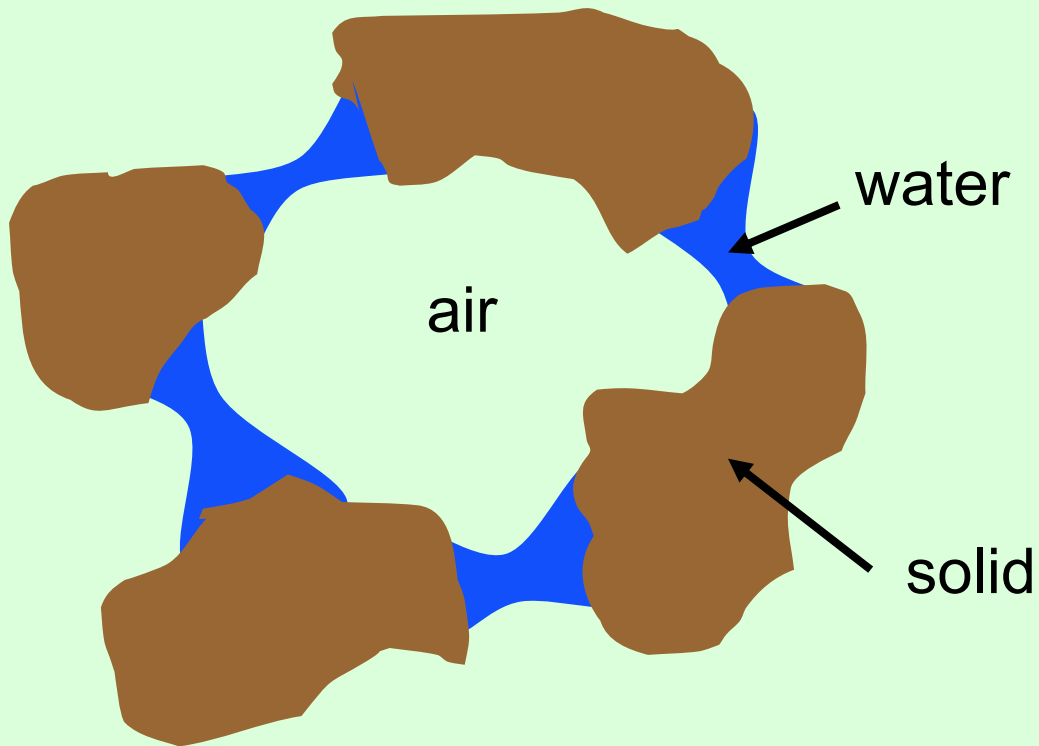
## **Contaminant Transport Mechanisms and Principles**

# BASIC DEFINITIONS



Capillary fringe may be >200 cm in fine silt  
In capillary fringe water is nearly saturated, but held in tension in soil pores

# MICRO VIEW OF UNSATURATED ZONE



Contaminant concentrations:

$C_w$ , mg/L  
concentration in water

$C_g$ , mg/L or ppmv  
concentration in gas

$C_s$ , gm/kg  
concentration in solids

# PARTITIONING RELATIONSHIPS

$$\text{Solid} \leftrightarrow \text{water} \quad \frac{C_s}{C_w} = K_d = \frac{\text{mg/kg solid}}{\text{mg/L water}}$$

$K_d$  = partition coefficient

$$\text{Water} \leftrightarrow \text{vapor} \quad \frac{C_g}{C_w} = H = \frac{\text{mol/m}^3 \text{ air}}{\text{mg/m}^3 \text{ water}}$$

H = Henry's Law constant

# HENRY'S LAW CONSTANT

H has dimensions: atm m<sup>3</sup> / mol

H' is dimensionless

$$H' = H/RT$$

R = gas constant =  $8.20575 \times 10^{-5}$  atm m<sup>3</sup>/mol °K

T = temperature in °K

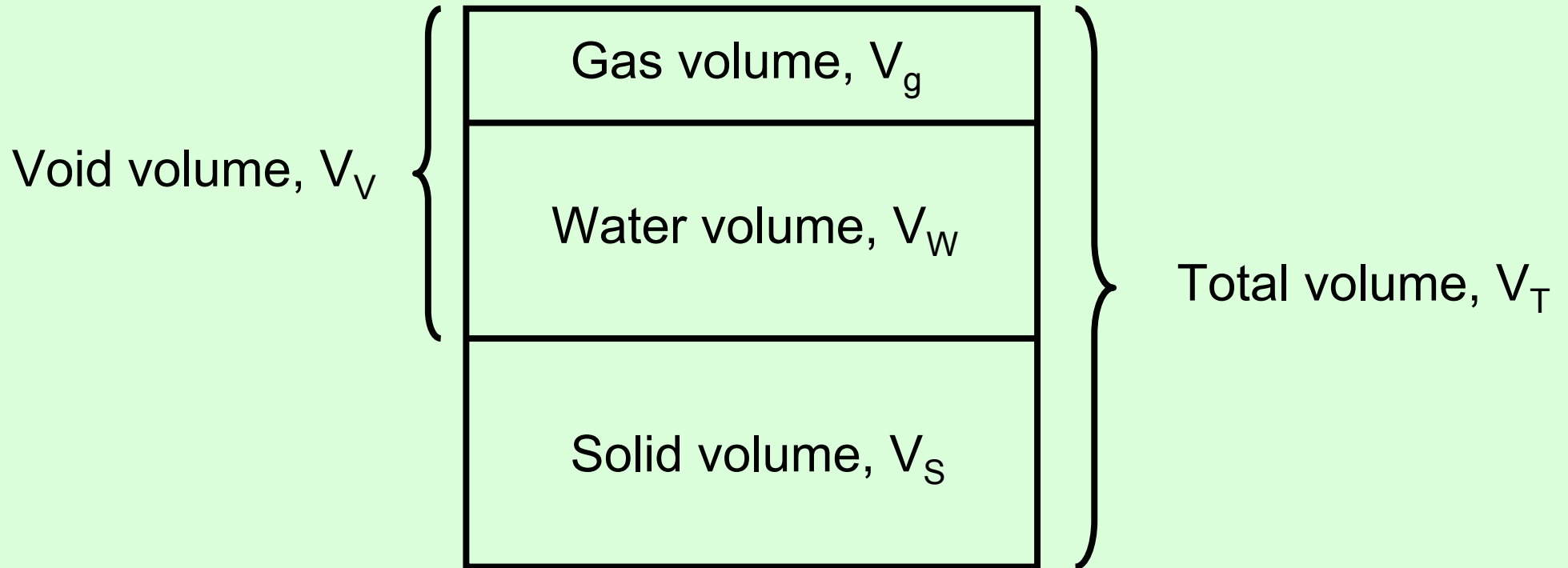
# NOTE ON SOIL GAS CONCENTRATION

Soil gas is usually reported as:

ppmv = parts per million by volume

$$C_g \text{ (ppmv)} = \frac{C_g \text{ (mg/L)} \times 24,000 \text{ mL/mole}}{\text{molecular weight g/mole}}$$

# VOLUME REPRESENTATION



# VOLUME-RELATED PROPERTIES

$$\text{Bulk density} = \rho_b = \frac{\text{mass of solids}}{\text{total volume}}$$

$$\text{Porosity} = n = \theta = V_v/V_T$$

$$\text{Volumetric water content or water-filled porosity} = \theta_w = V_w/V_T$$

$$\text{Saturation} = S = V_w/V_v$$

$$\text{Gas-filled porosity} = \theta_g \text{ (or } \theta_a) = V_g/V_T$$

$$\theta_w + \theta_g = n$$



# CONTAMINANT CONCENTRATION IN SOIL

Total mass in unit volume of soil:

$$C_T = \rho_b C_s + \theta_w C_w + \theta_g C_g$$

If soil is saturated,  $\theta_g = 0$  and  $\theta_w = n$

$$C_T = \rho_b C_s + n C_w$$

# NOMENCLATURE FOR DARCY'S LAW

$$Q = K i A$$

$K$  = hydraulic conductivity

$i$  = hydraulic gradient =  $dh/dL$

$A$  = cross-sectional area

Velocity of ground-water movement

$u = Q / n A = q / n = K i / n =$  average linear velocity

$n A =$  area through which ground water flows

$q = Q / A =$  Darcy seepage velocity = Specific discharge

For transport,  $n$  is  $n_e$ , effective porosity

# ADVECTIVE FLUX

Flowing ground water carries any dissolved material with it → Advective Flux

$$J_A = n u C \quad \text{mass / area / time}$$

= mass flux through unit cross section due to ground-water advection

n is needed since no flow except in pores

# DIFFUSIVE FLUX

Movement of mass by molecular diffusion (Brownian motion) – proportional to concentration gradient

$$\mathbf{J}_D = -D_o \frac{\partial C}{\partial \mathbf{x}} \quad \text{in surface water !!!}$$

$D_o$  is molecular diffusion coefficient [ $L^2/T$ ]

# DIFFUSIVE FLUX

In porous medium, geometry imposes constraints:

$$\mathbf{J}_D = -\tau \mathbf{D}_o \mathbf{n} \frac{\partial \mathbf{C}}{\partial \mathbf{x}} = -\mathbf{D}^* \mathbf{n} \frac{\partial \mathbf{C}}{\partial \mathbf{x}}$$

$\tau$  = tortuosity factor

$\mathbf{D}^*$  = effective diffusion coefficient

Factor  $n$  must be included since diffusion is only in pores

# TORTUOSITY

Solute must travel a tortuous path, winding through pores and around solid grains

Common empirical expression:  $\tau = \left( \frac{L}{L_e} \right)^2$

$L$  = straight-line distance

$L_e$  = actual (effective) path

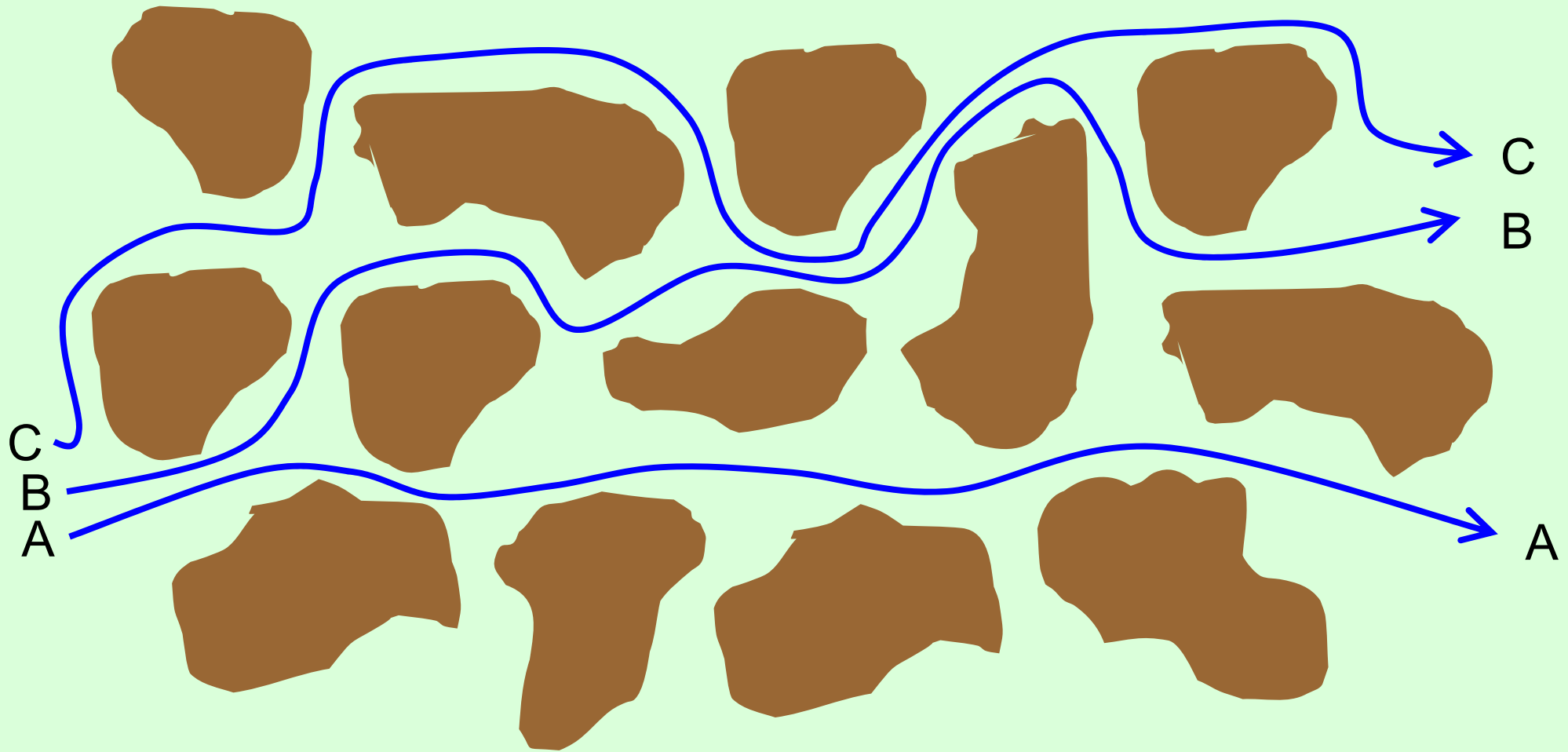
$\tau \approx 0.7$  for sand

# NOTES ON DIFFUSION

Diffusion is not a big factor in saturated groundwater flow – dispersion dominates diffusion

Diffusion can be important (even dominant) in vapor transport in unsaturated zone

# MECHANICAL DISPERSION



A arrives first, then B, then C → mechanical dispersion



# MECHANICAL DISPERSION

Viewed at micro-scale (i.e., pore scale) arrival times A, B, and C can be predicted

Averaging travel paths A, B, and C leads to apparent spreading of contaminant about the mean

Spatial averaging → dispersion

# MECHANICAL DISPERSION

Dispersion can be effectively approximated by the same relationship as diffusion—i.e., that flux is proportional to concentration gradient:

$$\mathbf{J}_M = -\mathbf{D}_M \mathbf{n} \frac{\partial \mathbf{C}}{\partial \mathbf{x}}$$

Dispersion coefficient,  $D_M = \alpha_L u$

$\alpha_L$  = longitudinal dispersivity (units of length)

# TRADITIONAL VIEW OF HYDRODYNAMIC DISPERSION

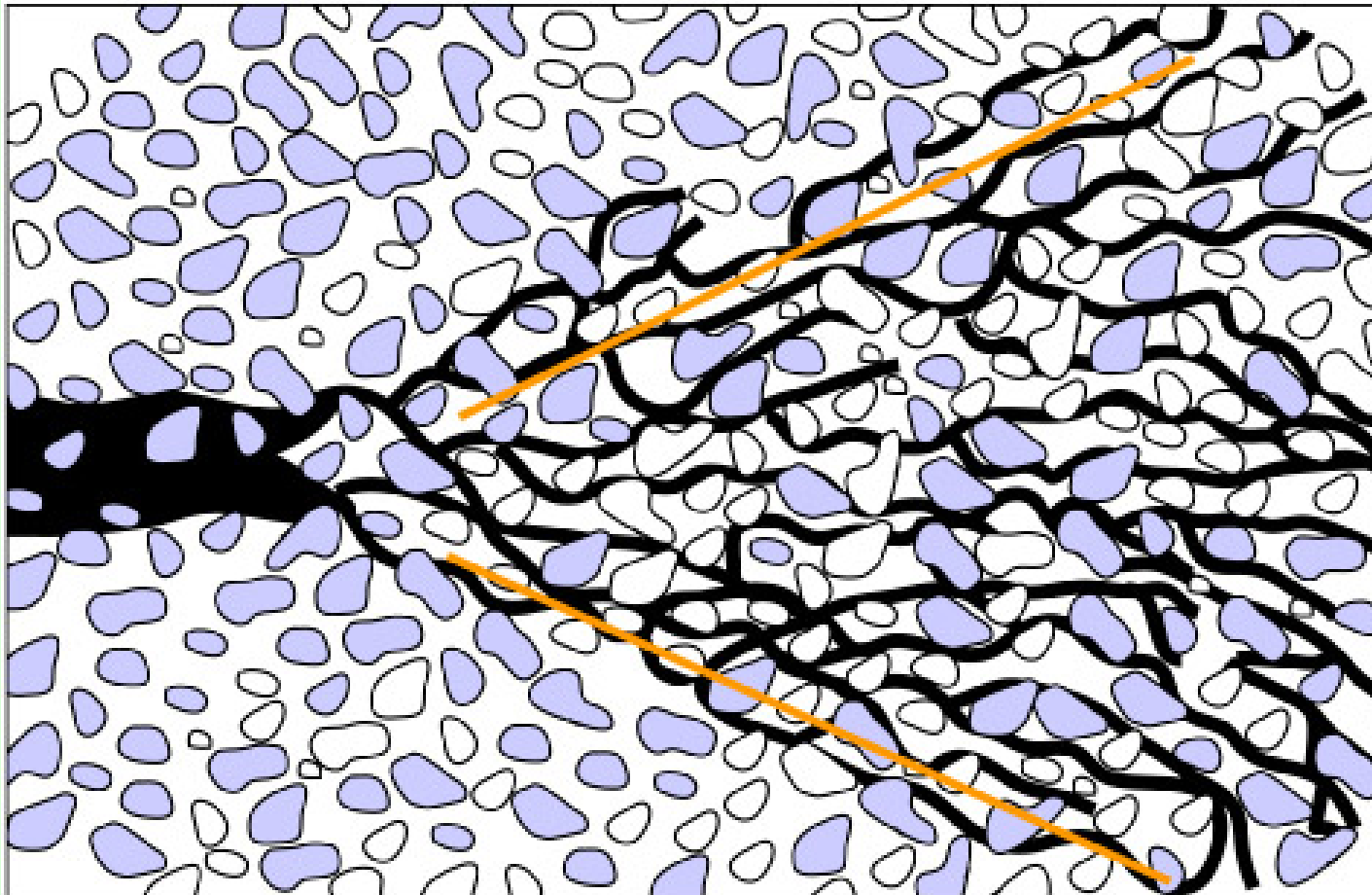


Image adapted from: Freeze, R.A., and J.A. Cherry, 1979. Groundwater. Prentice Hall, Englewood Cliffs, New Jersey. Pg. 384.

# ACTUAL OBSERVATIONS OF PLUMES

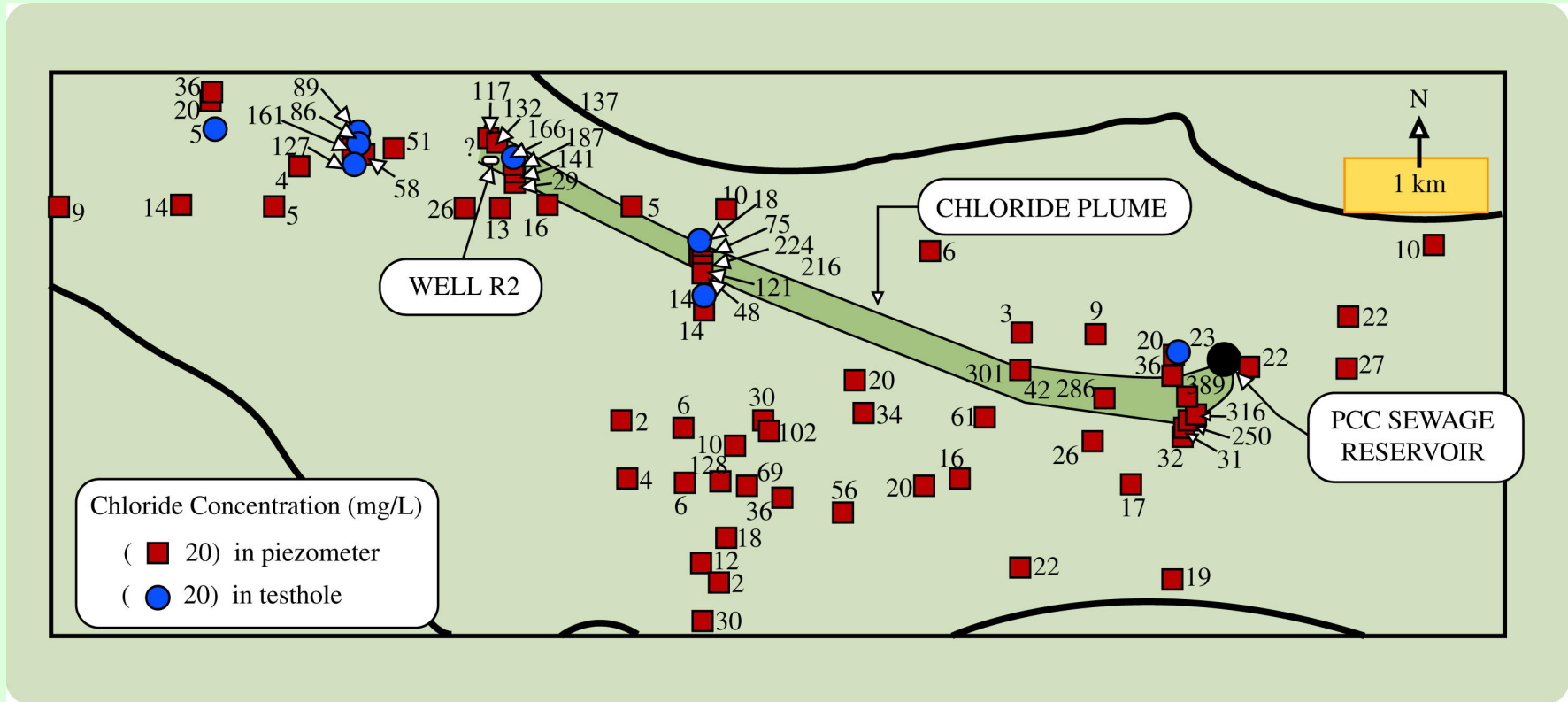
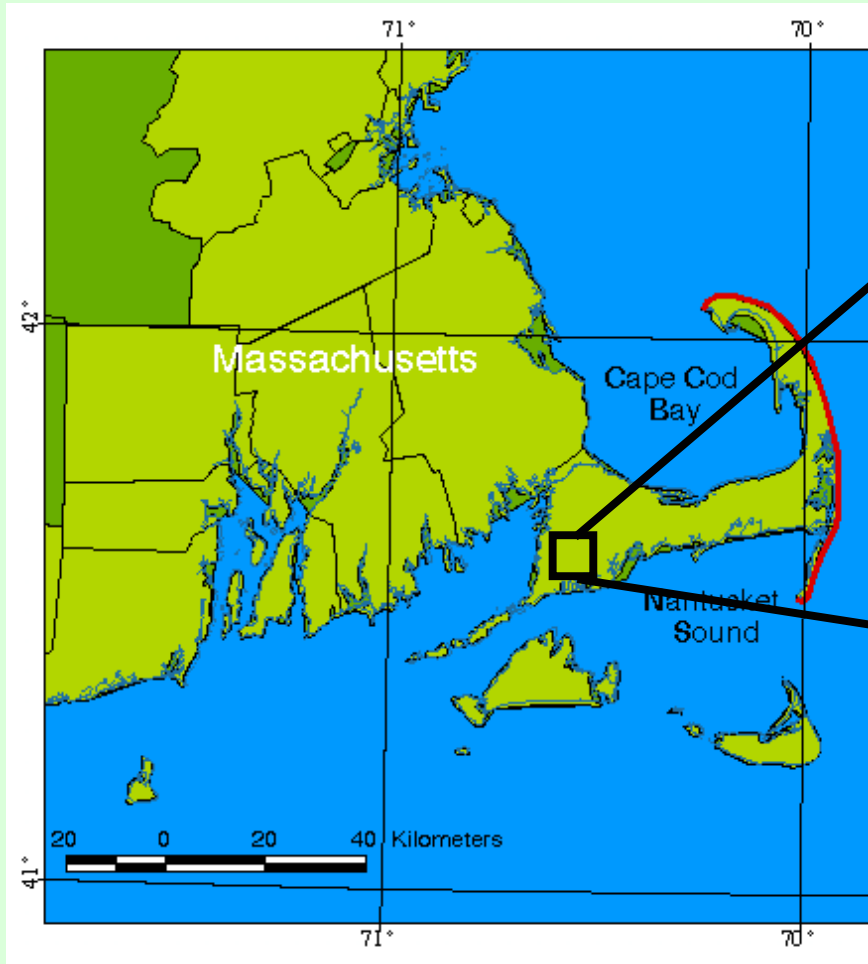
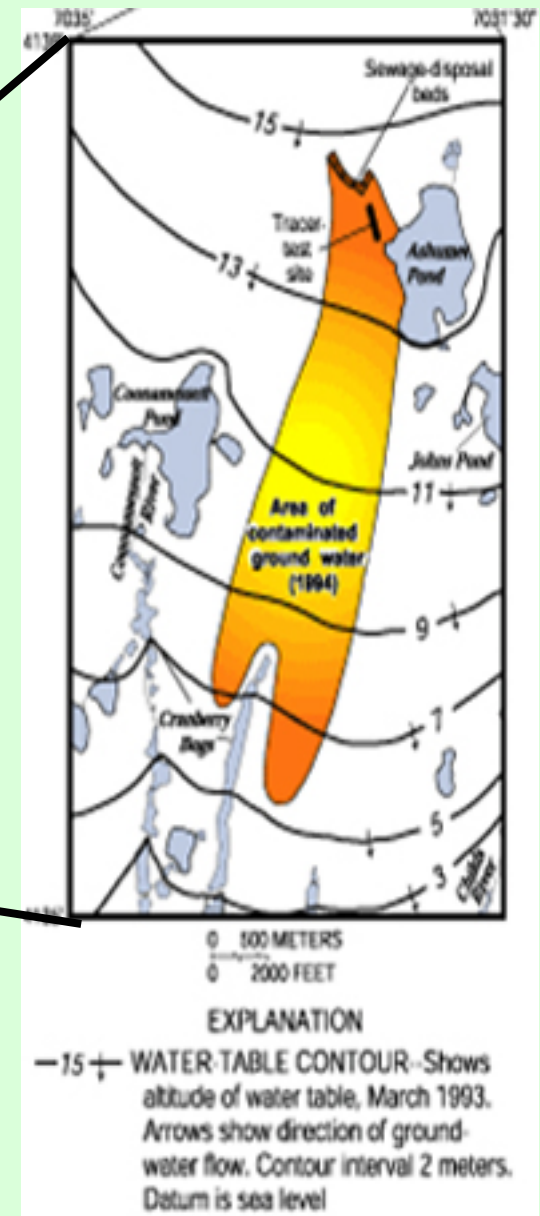


Image adapted from: Kamp, G.v.d., L.D. Luba, J.A. Cherry, and H. Maathuis, 1994. Field Study of a Long and Very Narrow Contaminant Plume. *Ground Water*. Vol. 32, No. 6, Pg. 1008. November/December 1994.

# USGS Cape Cod Research Site

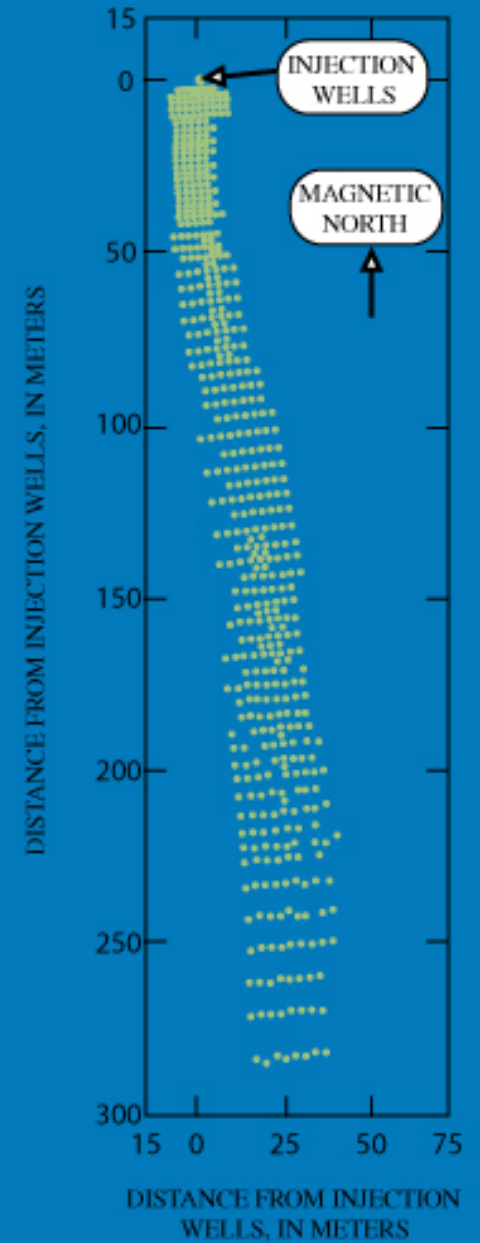


Source: NOAA Coastal Services Center,  
[http://www.csc.noaa.gov/crs/tcm/98fall\\_status.html](http://www.csc.noaa.gov/crs/tcm/98fall_status.html)  
Accessed May 14, 2004.



Source: U.S. Geological Survey, Cape Cod Toxic Substances Hydrology Research Site,  
<http://ma.water.usgs.gov/CapeCodToxics/location.html>  
Accessed May 14, 2004.

# MONITORING WELL ARRAY



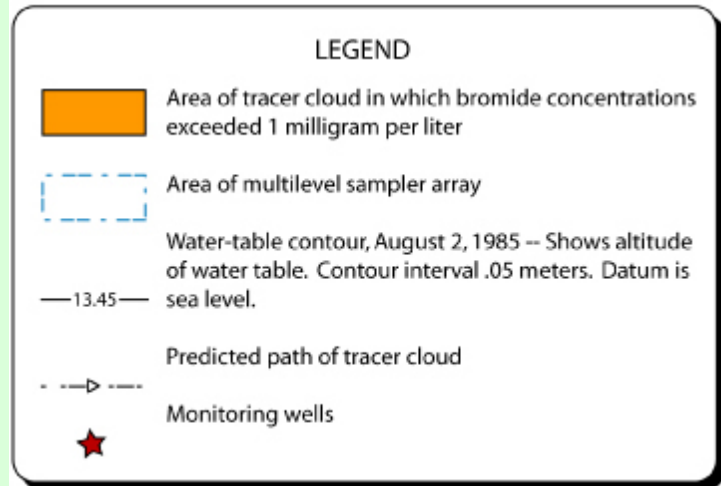
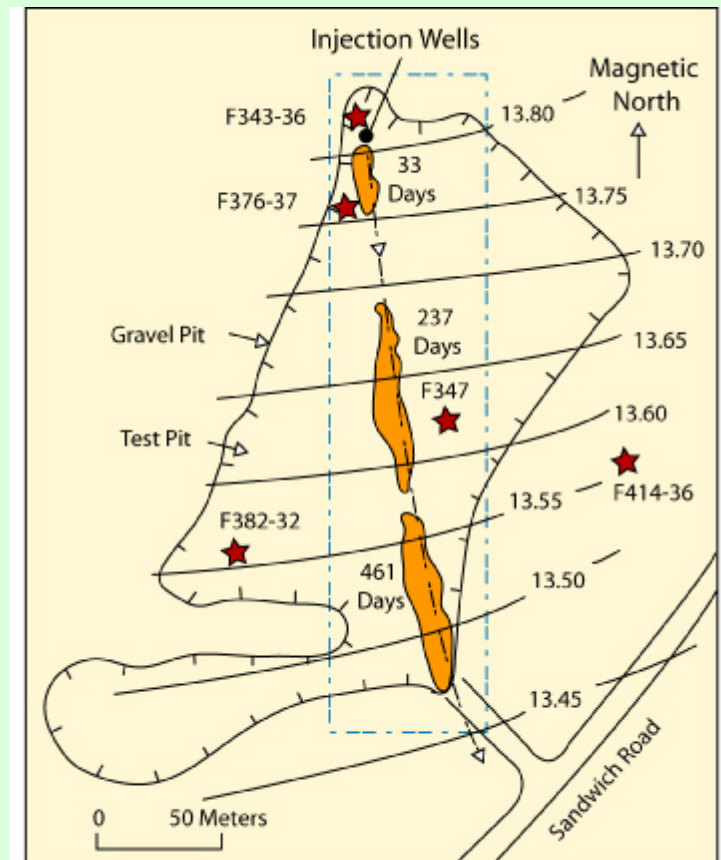


# USGS MONITORING NETWORK



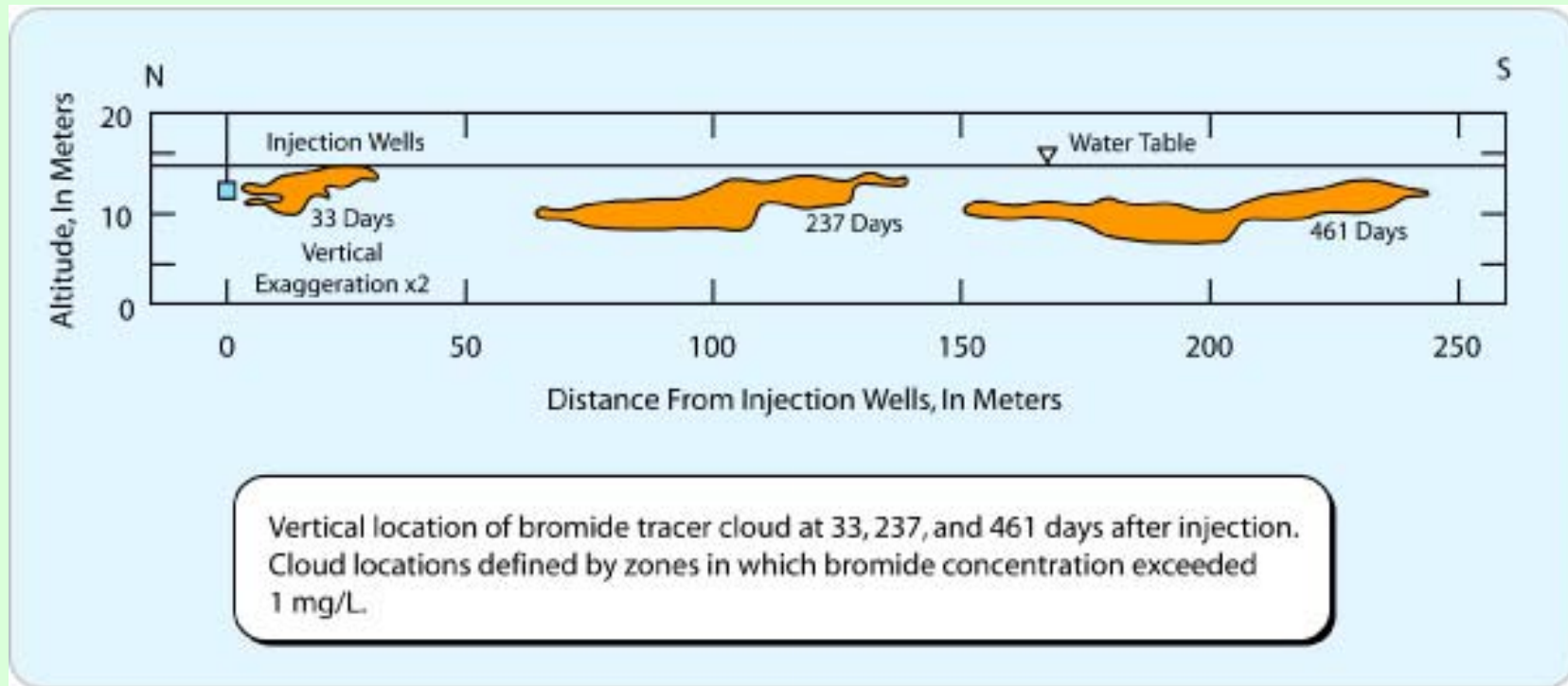
# OBSERVED BROMIDE PLUME – HORIZONTAL VIEW

Significant longitudinal  
dispersion, but limited  
lateral dispersion





# OBSERVED BROMIDE PLUME – VERTICAL VIEW



Limited vertical dispersion

# LONGITUDINAL DISPERSION VS. LENGTH SCALE

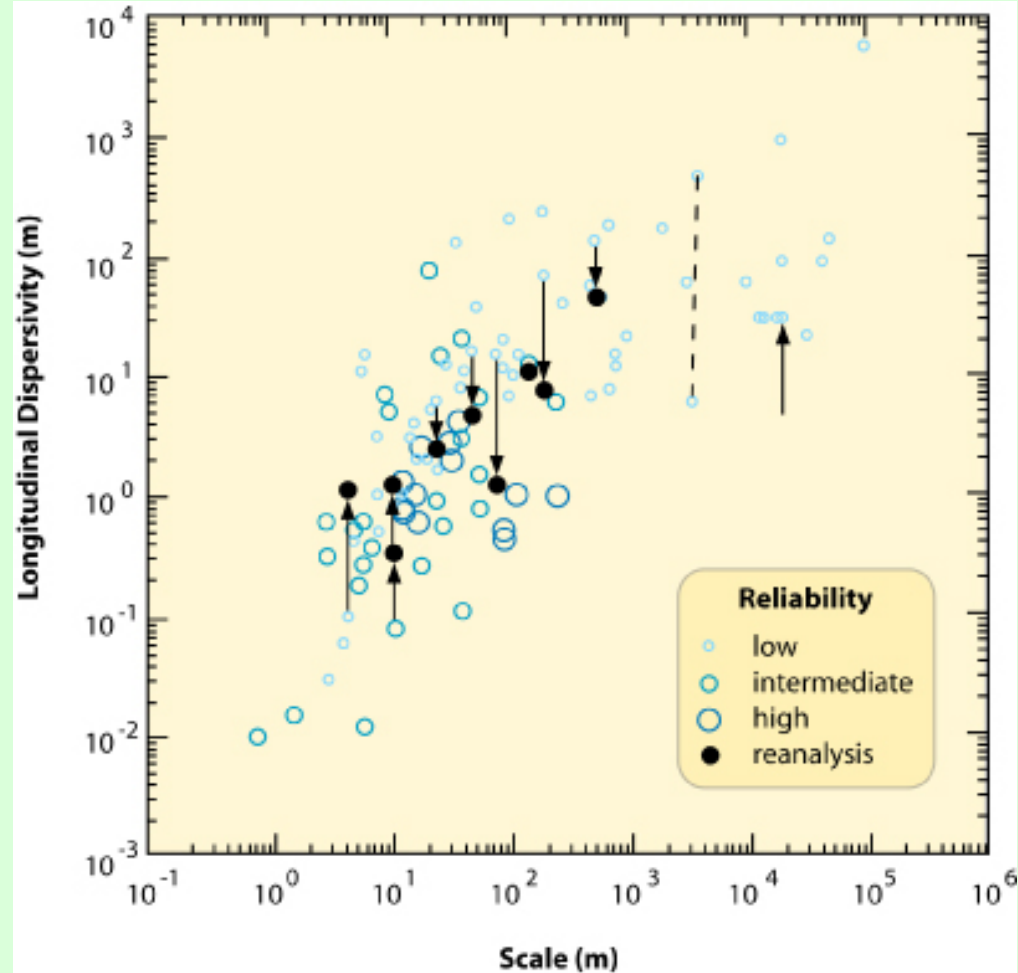


Image adapted from: Gelhar, L.W., C. Welty, and K.R. Rehfeldt, 1992. A critical review of data on field-scale dispersion in aquifers. *Water Resources Research*. Vol. 28, No. 7, Pg. 1955. July 1992.

# Lateral and vertical dispersivity

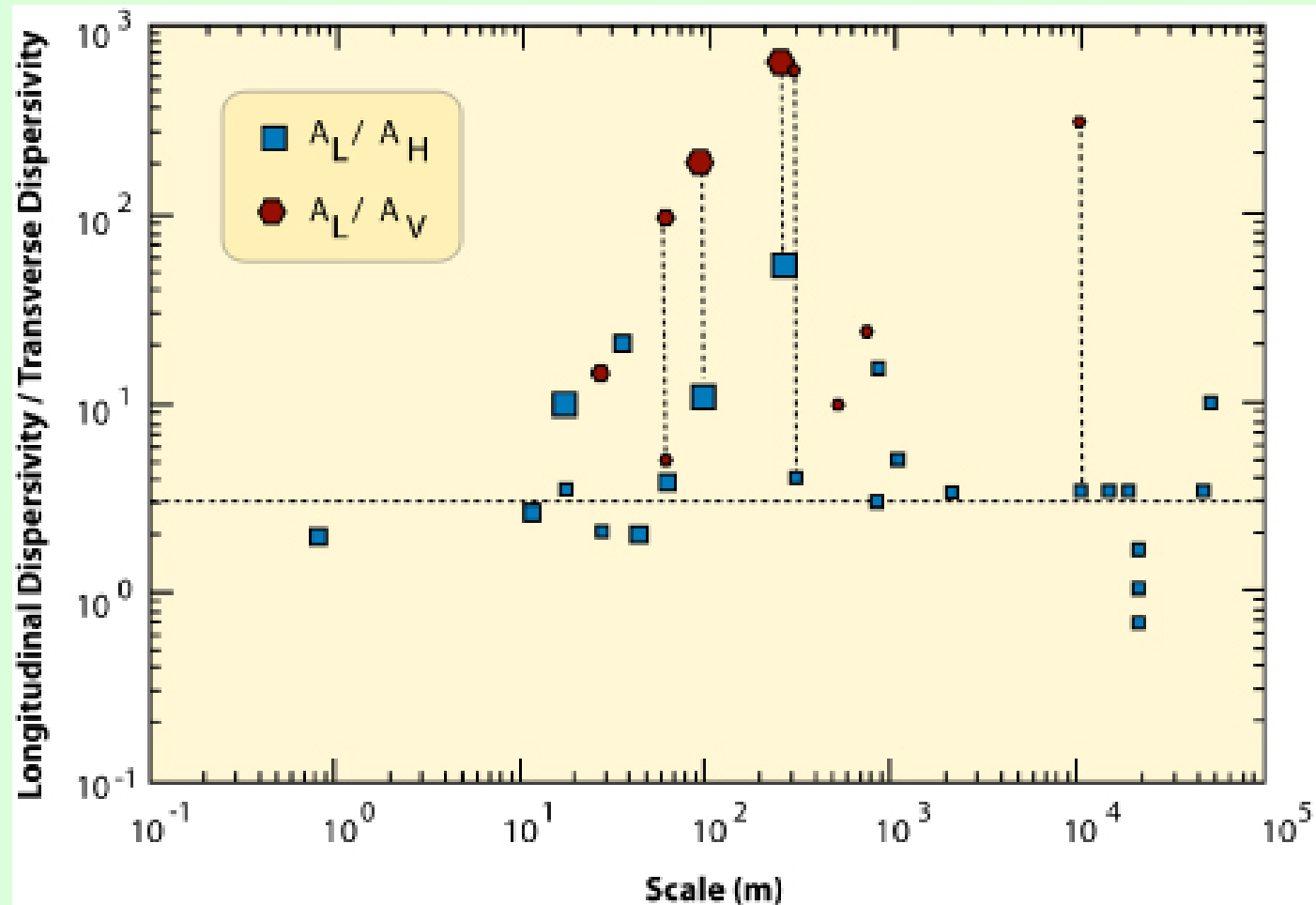


Image adapted from: Gelhar, L.W., C. Welty, and K.R. Rehfeldt, 1992. A critical review of data on field-scale dispersion in aquifers. *Water Resources Research*. Vol. 28, No. 7, Pg. 1955. July 1992.

# TRANSPORT EQUATION

Combined transport from advection, diffusion, and dispersion (in one dimension):

$$J = J_A + J_D + J_M$$

$$J = nuC - D^* n \frac{\partial C}{\partial x} - D_M n \frac{\partial C}{\partial x}$$

$$J = nuC - D_H \frac{\partial C}{\partial x}$$

$$\begin{aligned} D_H &= D^* + D_M = \tau D_O + \alpha_L u \\ &= \text{hydrodynamic dispersion} \end{aligned}$$

# TRANSPORT EQUATION

Consider conservation of mass over control volume (REV) of aquifer.

REV = Representative Elementary Volume

REV must contain enough pores to get a meaningful representation (statistical average or model)

# TRANSPORT EQUATION

Change in  
contaminant  
mass with  
time

Flux in less  
flux out of  
REV

Sources and  
sinks due to  
reactions

$$\frac{\partial \mathbf{C}_T}{\partial t} = -\nabla \cdot \mathbf{J} \pm \mathbf{S/S} \quad (1)$$

$$\frac{\partial \mathbf{C}_T}{\partial t} = -\frac{\partial \mathbf{J}}{\partial \mathbf{x}} \pm \mathbf{S/S} \quad (2)$$

# TRANSPORT EQUATION

$$\begin{aligned} C_T &= \text{total mass (dissolved mass plus mass adsorbed to} \\ &\quad \text{solid) per unit volume} \\ &= \rho_b C_S + n C_W = \rho_b C_S + n C \end{aligned} \quad (3)$$

Note: W subscript dropped for convenience and for  
Consistency with conventional notation

Substitute Equation 3 into Equation 2:

$$\frac{\partial(\rho_b \mathbf{C}_S)}{\partial t} + \frac{\partial(n\mathbf{C})}{\partial t} = -\frac{\partial}{\partial \mathbf{x}} \left( n\mathbf{u}\mathbf{C} - \mathbf{D}_H n \frac{\partial \mathbf{C}}{\partial \mathbf{x}} \right) \pm \mathbf{S} / \mathbf{S} \quad (4)$$

↑ no solid phase in flux term

# TRANSPORT EQUATION

$C_S = K_d C$  by definition of  $K_d$

Assume spatially uniform  $n$ ,  $\rho_b$ ,  $K_d$ ,  $u$ , and  $D_H$  and no S/S

$$(\rho_b K_d + n) \frac{\partial C}{\partial t} = -nu \frac{\partial C}{\partial x} + nD_H \frac{\partial^2 C}{\partial x^2} \quad (5)$$

$$\frac{\partial C}{\partial t} = - \frac{u}{\left( \frac{\rho_b K_d + n}{n} \right)} \frac{\partial C}{\partial x} + \frac{D_H}{\left( \frac{\rho_b K_d + n}{n} \right)} \frac{\partial^2 C}{\partial x^2} \quad (6)$$



# TRANSPORT EQUATION

“Retardation factor”,  $R_d$

$$\frac{\rho_b \mathbf{K}_d + \mathbf{n}}{\mathbf{n}} = 1 + \frac{\rho_b \mathbf{K}_d}{\mathbf{n}} = \mathbf{R}_d \quad (7)$$

Substituting Equation 7 into Equation 6:

$$\frac{\partial \mathbf{C}}{\partial \mathbf{t}} = - \frac{\mathbf{u}}{\mathbf{R}_d} \frac{\partial \mathbf{C}}{\partial \mathbf{x}} + \frac{\mathbf{D}_H}{\mathbf{R}_d} \frac{\partial^2 \mathbf{C}}{\partial \mathbf{x}^2} \quad (8)$$

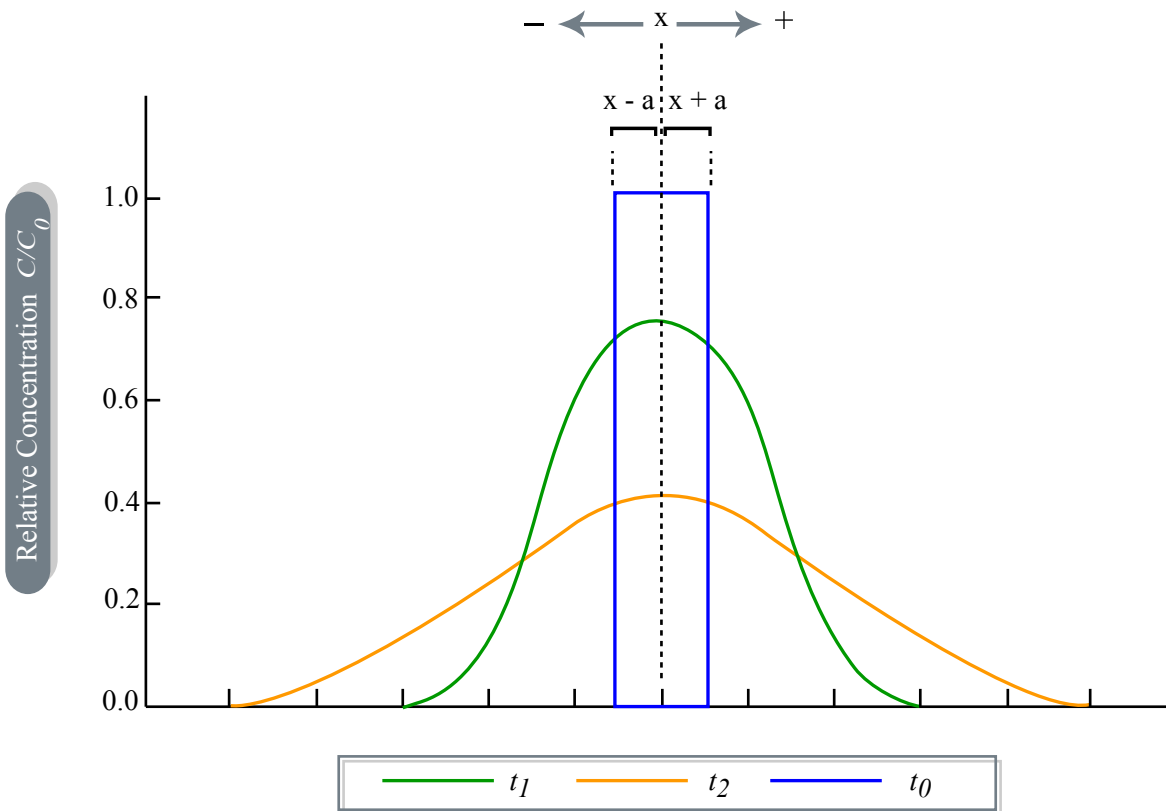
Effect of adsorption to solids is an apparent slowing of transport of dissolved contaminants

Both  $u$  and  $D_H$  are slowed

# SOLUTION OF TRANSPORT EQUATION

Equation 8 can be solved with a variety of boundary conditions

In general, equation predicts a spreading Gaussian cloud



Spreading of a solute slug with time due to diffusion. A slug of solute was injected into the aquifer at time  $t_0$  with a resulting initial concentration of  $C_0$ .

Adapted from: Fetter, C. W. *Contaminant Hydrogeology*.  
New York: Macmillan Publishing Company, 1992.

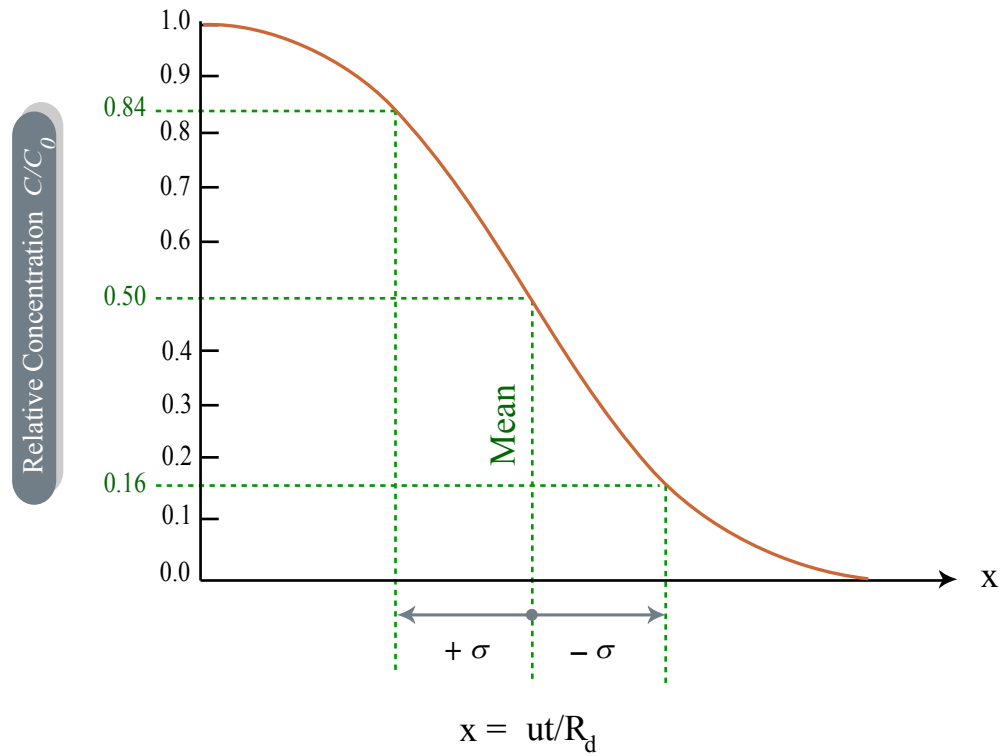
# 1-D SOLUTION OF TRANSPORT EQUATION

For instantaneous placement of a long-lasting source (for example, a spill that leaves a residual in the soil), solution is:

$$C(x, t) = \frac{C_o}{2} \operatorname{erfc} \left( \frac{R_d x - ut}{\sqrt{4R_d D_H t}} \right)$$

Where  $C_o = C(x=0, t) =$  constant concentration at source location  $x = 0$

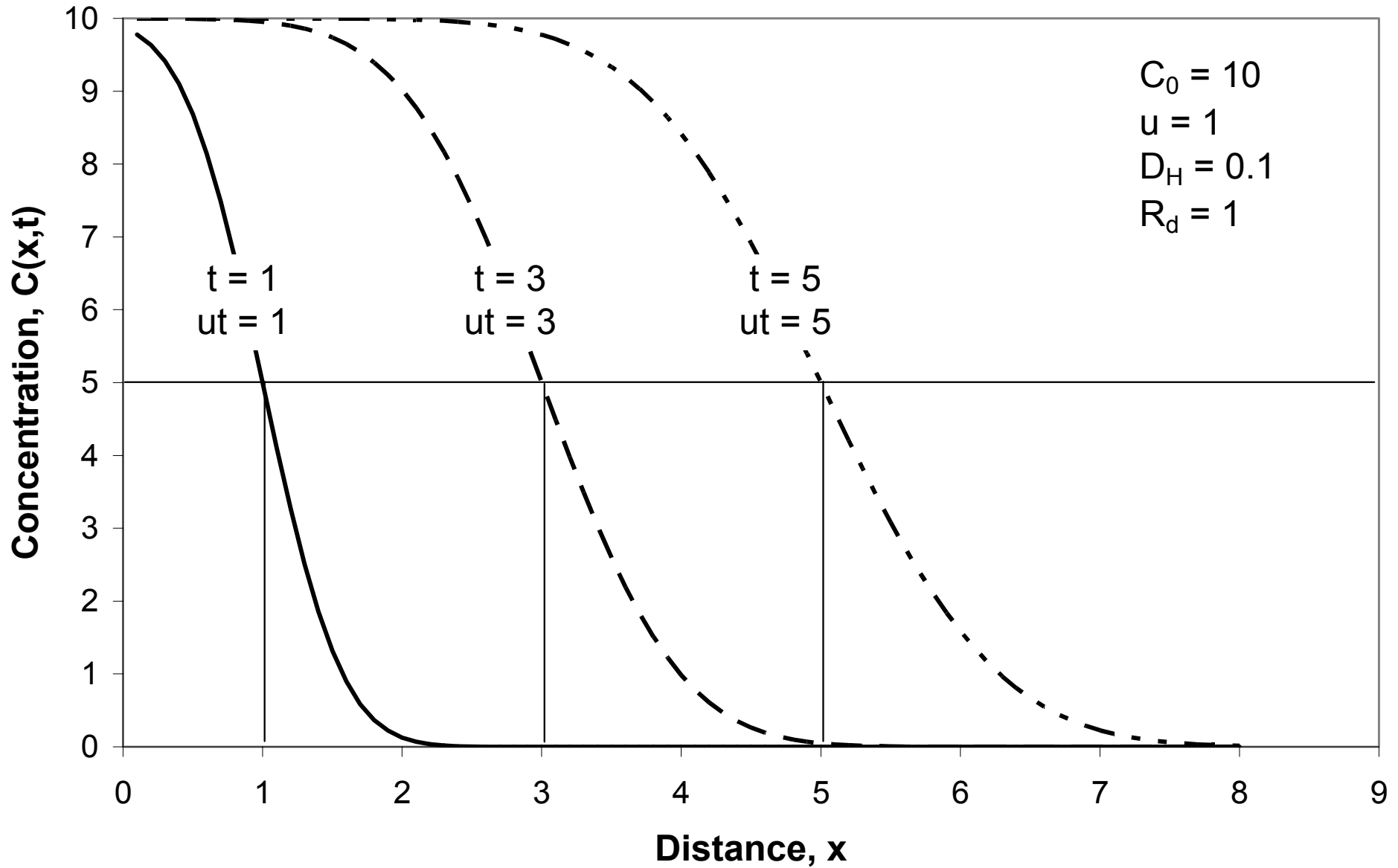
Solution is a front moving with velocity  $u/R_d$



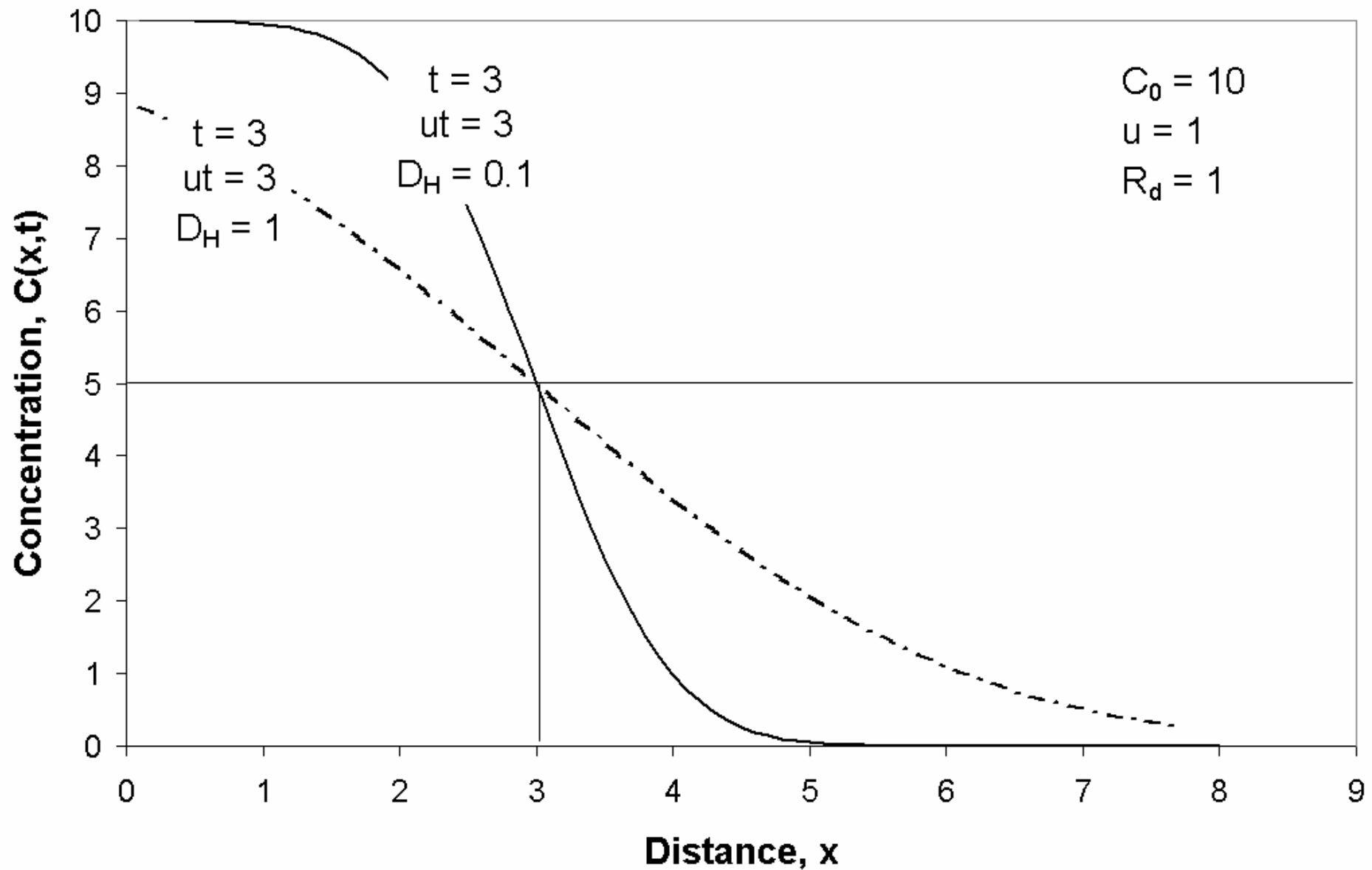
The profile of a diffusing front as predicted by the complementary error function.

Adapted from Fetter, C. W. *Contaminant Hydrogeology*.  
New York: Macmillan Publishing Company, 1992.

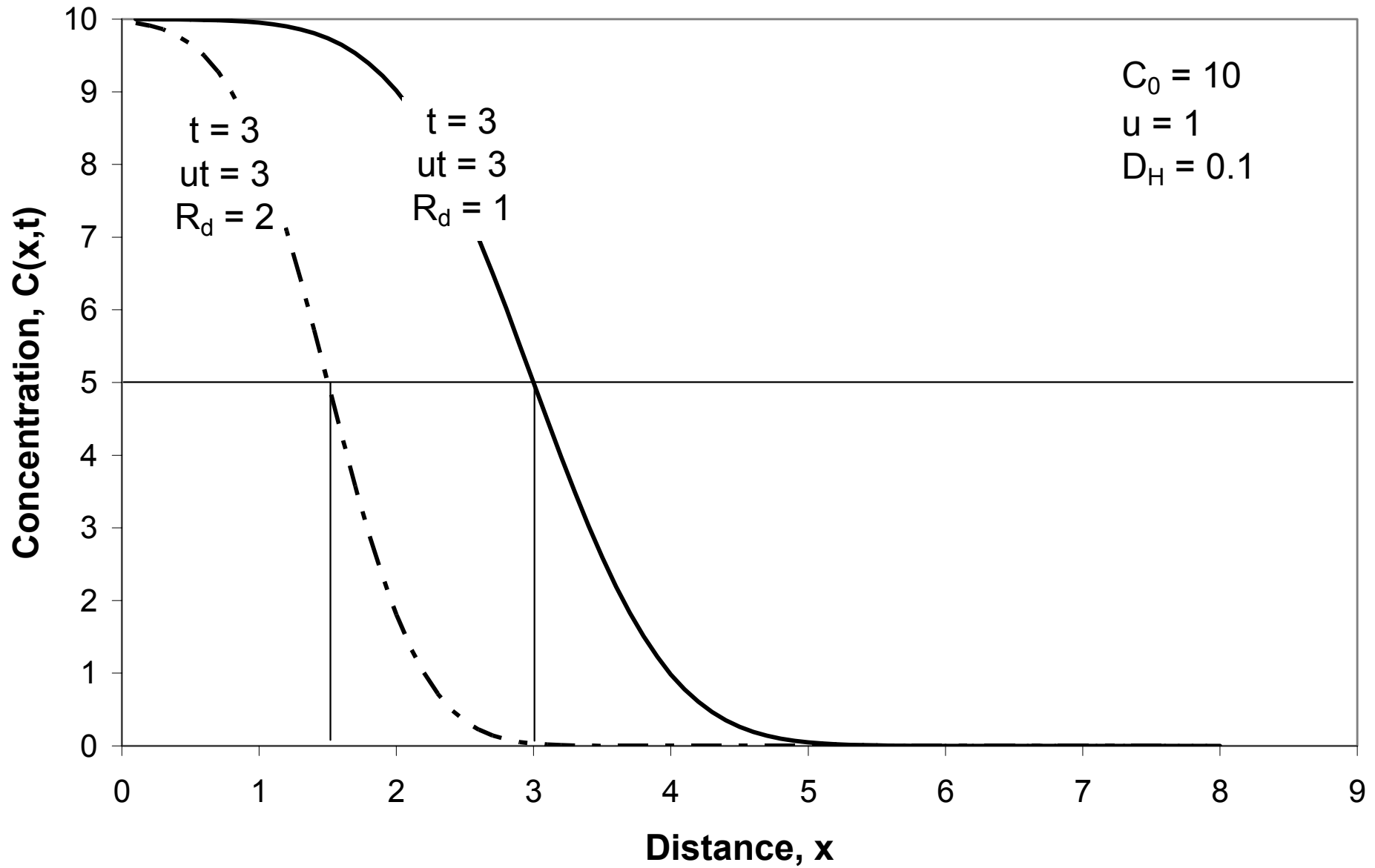
# Moving front of contaminant from constant source



## Effect of $D_H$ on moving front of contaminant



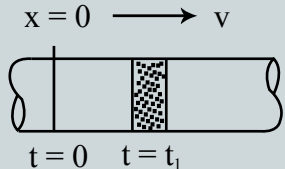
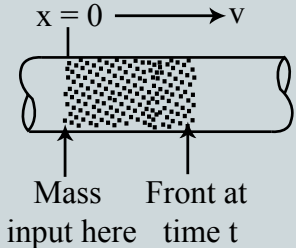
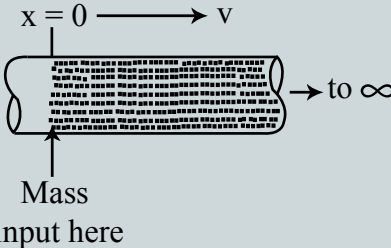
# Effect of $R_d$ on moving front of contaminant





# 1-D SOLUTIONS

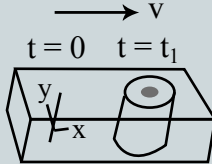
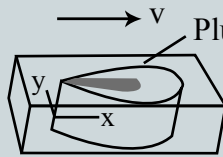
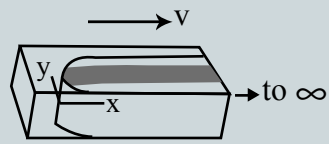
## Transport of a Conservative Substance from Pulse and Continuous Sources

Dimensions	Pulse Input of Mass $M$	Continuous Input of Mass Per Unit Time $\dot{M}$ Starting at Time $t = 0$	Continuous Input of Mass Per Unit Time $\dot{M}$ in Steady State
<p>1-D  <math>M, \dot{M}</math> are instantaneous or continuous plane sources</p> $M \left[ \frac{M}{L^2} \right]$ $\dot{M} \left[ \frac{M}{L^2 T} \right]$	$C = \frac{M}{2n\pi^{1/2} t^{1/2} \sqrt{D_x}} \exp\left[-\frac{(x-vt)^2}{4D_x t}\right]$ 	$C = \frac{\dot{M}}{2nv} \operatorname{erfc}\left(\frac{x-vt}{2\sqrt{D_x t}}\right)$ 	$C = \frac{\dot{M}}{nv} \quad (\text{for } x > 0)$ 

Adapted from: Hemond, H. F. and E. J. Fechner-Levy. *Chemical Fate and Transport in the Environment*. 2nd ed. San Diego: Academic Press, 2000.

# 2-D SOLUTIONS

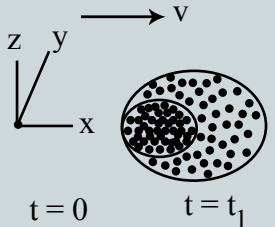
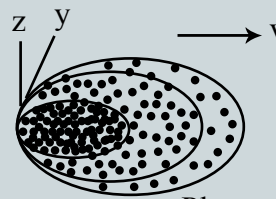
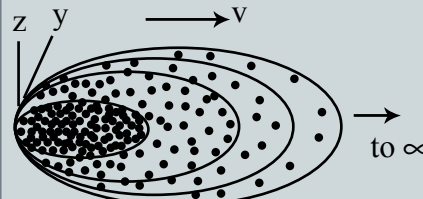
## Transport of a Conservative Substance from Pulse and Continuous Sources

Dimensions	Pulse Input of Mass $M$	Continuous Input of Mass Per Unit Time $\dot{M}$ Starting at Time $t = 0$	Continuous Input of Mass Per Unit Time $\dot{M}$ in Steady State
<p>2-D  <math>M, \dot{M}</math> are instantaneous or continuous line sources</p> <p><math>M \left[ \frac{M}{L} \right]</math></p> <p><math>\dot{M} \left[ \frac{M}{L-T} \right]</math></p>	$C = \frac{M}{4n\pi t \sqrt{D_x D_y}} \exp\left[-\frac{(x-vt)^2}{4D_x t} - \frac{y^2}{4D_y t}\right]$ 	$C = \frac{\dot{M}}{4n\pi^{1/2} (vr)^{1/2} \sqrt{D_y}} \exp\left[\frac{(x-r)v}{2D_x}\right] \operatorname{erfc}\left(\frac{r-vt}{2\sqrt{D_x t}}\right)$ 	$C = \frac{\dot{M}}{2n\pi^{1/2} (vr)^{1/2} \sqrt{D_y}} \exp\left[\frac{(x-r)v}{2D_x}\right]$ 

Adapted from: Hemond, H. F. and E. J. Fechner-Levy. *Chemical Fate and Transport in the Environment*. 2nd ed. San Diego: Academic Press, 2000.

# 3-D SOLUTIONS

## Transport of a Conservative Substance from Pulse and Continuous Sources

Dimensions	Pulse Input of Mass $M$	Continuous Input of Mass Per Unit Time $\dot{M}$ Starting at Time $t = 0$	Continuous Input of Mass Per Unit Time $\dot{M}$ in Steady State
<p>3-D  <math>M, \dot{M}</math> are instantaneous or continuous point sources</p> <p><math>M \left[ \frac{M}{L} \right]</math>  <math>\dot{M} \left[ \frac{M}{T} \right]</math></p>	$C = \frac{M}{8n\pi^{3/2} t^{3/2} \sqrt{D_x D_y D_z}} \exp\left[-\left(\frac{(x-vt)^2}{4D_x t} + \frac{y^2}{4D_y t} + \frac{z^2}{4D_z t}\right)\right]$  <p><math>t = 0</math>      <math>t = t_1</math></p>	$C = \frac{\dot{M}}{8n\pi r \sqrt{D_y D_z}} \exp\left[\frac{(x-r)v}{2D_x}\right] \operatorname{erfc}\left(\frac{r-vt}{2\sqrt{D_x t}}\right)$  <p>Plume at time <math>t</math></p>	$C = \frac{\dot{M}}{4n\pi r \sqrt{D_y D_z}} \exp\left[\frac{(x-r)v}{2D_x}\right]$  <p>to <math>\infty</math></p>

Adapted from: Hemond, H. F. and E. J. Fechner-Levy. *Chemical Fate and Transport in the Environment*. 2nd ed. San Diego: Academic Press, 2000.